1 Criticality in the planform behavior of the Ganges River

- 2 meanders
- 3 P.A. Carling¹, N. Gupta², P.M. Atkinson^{1,3}, and Huang Qing He⁴
- 4 ¹Geography and Environment Department, University of Southampton, Southampton
- 5 SO17 1BJ, UK
- 6 ²Tea Research Association, Jorhat 785008, India
- 7 ³Faculty of Science and Technology, Lancaster University, Lancaster LA1 4YW, UK

 \mathcal{O}

8 ⁴Chinese Academy of Sciences, 100864 Beijing, China

9 ABSTRACT

10 The critical point of planform transition from straight to meandering in the 11 wandering Ganges River is identifiable. Recent remote-sensing data indicate that four 12 similar meanders cut off, or attempted to cut off, after $\sim 31-35$ yr, primarily due to 13 channel aggradation. As main channels aggrade, sinuosity is maximized for broad 14 channel widths and small radii of curvature and relaxes for bends of greater radii. 15 Maximized form resistance occurs close to self-organized criticality and promotes 16 cutoffs. Avulsions lead to main channel narrowing and prevent further bend 17 tightening, relaxing the system by reducing sinuosity. Thus, the wandering river 18 oscillates in space and time across the transition from a more ordered to a more 19 chaotic state. Planform behavior is described by the Jerolmack-Mohrig mobility 20 number and the Parker stability criterion, which well define meander behavior as they 21 approach criticality and then relax via partial or completed avulsions. The results have 22 significance for river engineering and river network and stratigraphic modeling. Such 23 an approach could be of practical value when predicting the behaviors of other major 24 wandering rivers.

25 **INTRODUCTION**

26	Stølum (1996) showed that channel sinuosity oscillates across a predictable
27	critical state mediated by local cutoff (avulsion) processes. Such an adjustment is a
28	form of self-organized criticality (SOC; Bak, 1996); when the critical state is reached,
29	meanders adjust to regain order before evolving further. Using the criticality concept,
30	we show that the course of the wandering Ganges River, India (study area:
31	24.459317°N, 88.103924°E; Fig. 1), oscillates in space and time from a more ordered
32	to a more chaotic state (Stølum, 1996), without change in the magnitude and
33	frequency of external forcing. However, the SOC environment and time scale can be
34	subject to local fixed controls (here bedrock pinch points) that condition SOC
35	behavior (Camazine et al., 2001). The low-sinuosity river (ordered state) increases its
36	sinuosity (chaotic state) until local bank instabilities, manifest as avulsions, lead to
37	channel shortening to reach a low sinuosity value again. Meander regrowth follows.
38	Thus, the critical state is defined as the planform pattern transition point.
39	Between Farakka Barrage (West Bengal, India) and Hardinge Bridge (Sara,
40	Bangladesh), three meanders occur, with a further meander immediately upstream of
41	the barrage (Fig. 1[[The figure does not identify the barrage or bridge – label
42	these in the figure? At any river kilometer, there is a low-gradient sandy main
43	meandering channel or up to three additional lesser cutoff channels. Such rivers are
44	termed "wandering" (Church, 1983). Floodplains and bars have no significant
45	vegetation control. Today, the basal control point of the upstream bend is the Farakka
46	Barrage, and at each of the other bends, translation is limited by geological pinch
47	points (Hossain et al., 2013) that impose important control on meander evolution.
48	Eleven maps (A.D. 1780–1967) reveal a persistent pattern of four meanders
49	increasing in amplitude without downstream translation until cutoffs occur over

50	decadal time scales that lead to periodic reduction in main channel length and
51	sinuosity. In addition, 38 yr of remote sensing data (Landsat Multispectral Scanner,
52	Thematic Mapper, Indian Remote Sensing Satellites Linear Imaging Self-Scanning
53	[LISS] I and LISS III) (from 1972) were used to explore channel planform changes by
54	identifying completed avulsions or partial avulsions (Fig. 1). Main channel widths and
55	radii of curvature at meander apices were quantified for each of the four meanders
56	through time.

57 SETTING

58 The annual peak flow on the Ganges River usually occurs within a 1.5 m stage 59 range. Bankfull discharge is exceeded yearly, then the low natural levees are 60 overtopped by shallow floodplain flow or are breached by small cutoffs that transect 61 the major meander loops. These cutoffs scour the floodplains (Coleman, 1969), but 62 the main channel does not realign. Rather, it takes several years for the main flow to 63 adopt any enlarging cutoff channel (Fig. 1). Upstream of the Farakka Barrage the sediment load is 729×10^6 t yr⁻¹ (Wasson, 2003) which, due to the barrage, reduces 64 downstream to $300-500 \times 10^6$ t yr⁻¹ at Hardinge Bridge (Hossain et al., 2013). The 65 66 barrage (constructed in 1975) was fully aggraded by 1995 (Fig. 2), and much 67 sediment now passes by canal to the Bhagirathi-Hooghly River. Thus, the sediment 68 load downstream of the barrage reduces by $\sim 41\% - 68\%$. 69 Four similar meander bends were studied (Fig. 1): one upstream (R1) and 70 three downstream (R2-R4) of the barrage. All bends developed simultaneously and 71 cut off, or attempted to cut off, by chute development over similar time scales (31–35 72 yr). Thus, although the remote sensing time series is too short to develop a statistical 73 assessment of cutoff frequency, there are four replicates of the cutoff phenomenon.

74 CONDITIONS FOR AVULSION

75	DOI:10.1130/G38382.1 The avulsion condition largely is due to channel aggradation (Jerolmack and
76	Mohrig, 2007) that forces overbank flows to occur more frequently. However,
77	tightening bends deepen on their outer banks (Seminara, 2006), and increasing bend
78	flow resistance causes both elevation in the outer bank flow level and increased bank
79	erosion, which increases [[Corre]] channel width (Germanoski and Schumm,
80	1993). These conditions jointly are conducive to avulsion. Thus, the critical cutoff
81	condition can be determined for each bend and depends on (1) channel geometry, (2)
82	discharge, and (3) aggradation rate.
83	Channel Geometry
84	The radius of curvature (r) was determined for each of the main channel
85	bends. The radii of curvature decreased through time, whereas the channel widths (B)
86	often increased (Hossain et al., 2013). The inability of point bar progradation to match
87	the rate of bend apex recession, such that B increases as bends tighten, has been noted
88	elsewhere (Kasvi et al., 2015). The condition preceding a completed (or attempted)
89	cutoff and a sudden decrease in sinuosity (S) occurred when the bend radius fell to
90	between 5000 m and 2000 m. Thus, cutoff likelihood, in part, can be defined by the
91	ratio r/B (Howard and Knutson, 1984). To cut off, the river must flow overbank and
92	avulse by rapid erosion of the levee and floodplain surface. The minimum condition
93	for overbank flow is bankfull discharge (van Dijk et al., 2014) plus super-elevated
94	outer bank flow. For bankfull flow ($Q_b \sim 56,633 \text{ m}^3 \text{ s}^{-1}$; Coleman, 1969), for the

channel width (~4000 m) immediately before cutoff occurs, and for the minimum

96 radius of curvature (2000 m), the water surface super-elevation (Δy) is:

97
$$\Delta y = \frac{c\overline{U}^2\overline{B}}{rg},$$
 (1)

98 where *c* is a coefficient (0.5) for subcritical flows, the bankfull bulk-flow velocity 99 $\overline{U} = Q_{\rm b}/\overline{h} \ \overline{B}$, where \overline{B} and \overline{h} are average values of the channel width and depth (*h*)

100	DOI:10.1130/G38382.1 at bankfull, and g is acceleration due to gravity. Bankfull velocity is low (on the order
101	of 1 m s ^{-1}) such that inertia is small. Thus, super-elevation at the bankline is no more
102	than -50 mm above the channel center water surface. So, for these shallow overbank
102	than ~50 min above the channel center water surface. So, for these sharlow overbank
103	conditions, near-bankfull flows alone are not likely to induce cutoff (Howard, 2009).
104	Rather, sustained outer-bank erosion, causing r/B to continue to decrease and further
105	channel aggradation, is required to elevate water levels additionally. Alternatively,
106	discharges much above bankfull are required.
107	Discharge
108	Rapid erosion of the outside bend will occur if discharge is adequate to entrain
109	bank material for a sufficient time (Edmonds et al., 2009). Bendway flow resistance
110	will reach a maximum as the radius of curvature reaches a minimum value. The
111	straight channel shear stress (τ_T) due to skin friction (<i>f</i>) is:
112	$\tau_{\rm T} = \rho g R S_{\rm e} = \rho f \overline{U}^2 , \qquad (2)$
113	where ρ is the density of water, R is the hydraulic radius, and S _e is the energy slope.
113 114	where ρ is the density of water, <i>R</i> is the hydraulic radius, and <i>S</i> _e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull <i>S</i> _e of 5–6 × 10 ⁻⁵ (Coleman,
113114115	where ρ is the density of water, <i>R</i> is the hydraulic radius, and <i>S</i> _e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull <i>S</i> _e of 5–6 × 10 ⁻⁵ (Coleman, 1969). These data provide an estimate of unit shear stress on the order of 10 N m ⁻² .
113114115116	where ρ is the density of water, <i>R</i> is the hydraulic radius, and S_e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull S_e of 5–6 × 10 ⁻⁵ (Coleman, 1969). These data provide an estimate of unit shear stress on the order of 10 N m ⁻² . Determining additional form resistance induced by bends is complex (e.g., Chang,
113114115116117	where ρ is the density of water, <i>R</i> is the hydraulic radius, and S_e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull S_e of 5–6 × 10 ⁻⁵ (Coleman, 1969). These data provide an estimate of unit shear stress on the order of 10 N m ⁻² . Determining additional form resistance induced by bends is complex (e.g., Chang, 1983). However, for illustrative purposes, we utilize the method of Leopold et al.
 113 114 115 116 117 118 	where ρ is the density of water, <i>R</i> is the hydraulic radius, and S_e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull S_e of 5–6 × 10 ⁻⁵ (Coleman, 1969). These data provide an estimate of unit shear stress on the order of 10 N m ⁻² . Determining additional form resistance induced by bends is complex (e.g., Chang, 1983). However, for illustrative purposes, we utilize the method of Leopold et al. (1960) to estimate bend form shear stress ($\tau_B = \rho g \bar{h} S_{\zeta}$) using an energy dissipation
 113 114 115 116 117 118 119 	where ρ is the density of water, <i>R</i> is the hydraulic radius, and S_e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull S_e of 5–6 × 10 ⁻⁵ (Coleman, 1969). These data provide an estimate of unit shear stress on the order of 10 N m ⁻² . Determining additional form resistance induced by bends is complex (e.g., Chang, 1983). However, for illustrative purposes, we utilize the method of Leopold et al. (1960) to estimate bend form shear stress ($\tau_B = \rho g \bar{h} S_{\zeta}$) using an energy dissipation term ($\bar{h}S_{\zeta}$):
 113 114 115 116 117 118 119 120 	where ρ is the density of water, <i>R</i> is the hydraulic radius, and S_e is the energy slope. The hydraulic radius is ~16 m with a regional bankfull S_e of 5–6 × 10 ⁻⁵ (Coleman, 1969). These data provide an estimate of unit shear stress on the order of 10 N m ⁻² . Determining additional form resistance induced by bends is complex (e.g., Chang, 1983). However, for illustrative purposes, we utilize the method of Leopold et al. (1960) to estimate bend form shear stress ($\tau_B = \rho g \bar{h} S_{\zeta}$) using an energy dissipation term ($\bar{h} S_{\zeta}$): $\bar{h} S_{\zeta} = \frac{\bar{U}^2}{g} (\frac{B}{r} - 0.5) - h(1 + 1.5F^{0.66})$, (3)

122 values of r/B, the form-induced shear stress can be up to an order of magnitude larger

123 than the skin shear stress. For greater r/B values, the form resistance declines. When

124	avulsions were imminent, values of r/B are consistent for all four reaches ($\overline{1.29}$,
125	standard deviation 0.72; $n = 27$) but smaller than those values (~3) reported by Begin
126	(1986) and Howard and Knutson (1984) for the condition when bank retreat through
127	erosion is maximized. Thus, the ability of the channel to develop significant form
128	resistance and adjust through increasing sinuosity is maximized for small radii of
129	curvature and decreases for bends of greater amplitude. However, increasing form
130	resistance as bends tighten induces a backwater effect and super-elevation that is
131	conducive to cutoff before r/B is maximized, preventing further bend tightening and
132	relaxing the system by reducing sinuosity.
133	Aggradation
134	The aggradation rates for meander bends R2–R4 are unknown, but for R1,
135	channel aggradation and subsequent attempted avulsion were induced by backwater
136	sedimentation above the barrage. A linear and then asymptotic approach to constant
137	zero aggradation is typical of impoundments (Wu et al., 2012) and provides a
138	maximum aggradation rate, $\sim 0.18 \text{ m yr}^{-1}$, to use as a scalar in R1 (Fig. 2A). Bend
139	extension increases rapidly once one-third of the impoundment depth is filled (Fig.
140	2B). For R2–R4, the aggradation rate (V_a) is assumed to be proportional to the
141	reduction in the sediment load ($V_a = 300/729 \times 0.18 \text{ m yr}^{-1}$) below the Farakka
142	Barrage. As the system aggraded, channel sinuosity increased, and attempted
143	avulsions and cutoffs developed (Figs. 2 and 3). As channel aggradation rate,
144	T_{A} [[Clarify how this differs fr V_{a}]], mediates the rate of lateral erosion, T_{C} , the
145	latter a key variable to define critical state (Stølum, 1998), consideration of $T_A:T_C$ can
146	define the critical state of the planform pattern transition if other factors are
147	significantly subordinate.
148	PLANFORM SCALING MODEL

Publisher: GSA Journal: GEOL: Geology DOI:10.1130/G38382.1 149 The model used to show the meander behavior is the Jerolmack and Mohrig (2007) approach to calculate the avulsion frequency (f_A) of a river. The avulsion 150 151 frequency, $f_{\rm A} = \frac{V_{\rm a}N}{\overline{h}}$, 152 (4) 153 is known approximately. Each reach avulsed, or tried to avulse, at a time scale of 154 ~31–35 yr, so f_A can be set to 0.03 for active channels N = 1-4, with an average 155 channel depth of $\overline{h} = 22$ m. Jerolmack and Mohrig (2007) developed a channel 156 mobility number (M) to discriminate single-channel versus multichannel form: $M = \frac{T_{\rm A}}{T_{\rm C}} = \frac{\overline{h}}{B} \frac{V_{\rm C}}{V_{\rm A}},$ 157 (5) where $T_{\rm C}$ is the time to migrate one channel \mathcal{D} th and $V_{\rm c}$ is the bank erosion rate. M =158 159 $T_{\rm A}/T_{\rm C}$ = 1 defines the critical planform pattern transition (Jerolmack and Mohrig, 160 2007). The general trend of M in Figure 3 shows the temporal trajectories of reach 161 behavior. For M >> 1, a single, laterally mobile sinuous channel is expected. For $M \approx$ 162 1, then transition is expected between a single channel and multiple channels. For M 163 << 1, a multichannel avulsive system is expected. In accord with SOC, few, small 164 avulsions release energy which suppresses the likelihood of large avulsions, whereas 165 large avulsions increase the energy capacity of the network, which is a destabilization 166 (Stølum, 1998). Accordingly, the network is attracted to $M \approx 1$. Such a simple model 167 uses few parameters to elucidate emergent behavior without appeal to detailed 168 process. 169 *M* is used here with the Parker (1976) channel stability criterion (ε),

170
$$\varepsilon = S_{\rm e} \sqrt{g\bar{h}\bar{B}^4}/Q, \qquad (6)$$

to define system trend through channel pattern phase space (Fig. 4), where Q is a
formative discharge (bankfull value). A single-thread channel should dominate when

173	$\varepsilon \ll 1$, while a braided form should be common for $\varepsilon \ge 1$. Jerolmack and Mohrig
174	(2007) argued that a plot of <i>M</i> versus ε discriminated between planforms representing
175	rivers at a single point in time across spatial scales. In contrast, we use the M - ε phase
176	space to explore meander bend evolutions through time as the channel morphology
177	varies across the point of criticality due to hydraulic and morphological forcing. It is
178	evident that meander R1 differs in its behavior in contrast to R2–R4, in that the Parker
179	criterion for R1 lies between values of 0.6 and 1.5 while the other meanders exhibit
180	values typically <0.4. The values of $M = 1$ and $\varepsilon < 0.4$ define four quadrant phase
181	spaces for channel planform discrimination (Fig. 4).
182	DISCUSSION
183	A power-law avulsion distribution may characterize SOC behavior but, as
184	with many studies (Hooke, 2007), our reach length is inadequate for this test. In
185	addition, a time constant is imposed on the Ganges' SOC cutoff behavior by spatial
186	pinch points, such that cycling occurs, similar to other guided SOC phenomena
187	(Prokopenko et al., 2014).
188	So, we focused on the critical state: defining avulsion as an autogenic response
189	of a channel when it cannot adjust further through gradual variation of sinuosity
190	(Stølum, 1996). As <i>M</i> approaches 1, there is an increased propensity for channel
191	alignment to reset by cutoff to regain low sinuosity.
192	In a flume, lacking bank-stabilizing vegetation, cutoffs occurred at a small
193	value of $S \approx 1.2$, preventing the development of more sinuous channels (Braudrick et
194	al., 2009). The Ganges River also is vegetation free and tends to avulse when S is ~ 1.3
195	(Fig. 3). However, the situation is not simple, as a new avulsion relaxes the system
196	such that both cutoff and main channel can be simultaneously active. There is not
197	usually a simple abandonment of the main channel in favor of the new channel (Fig.

198	1). These "soft avulsions" (Edmonds et al., 2011) divert some discharge and sediment
199	from the main channel (Coleman, 1969), but much load continues down the main
200	channel. The effects of cutoffs on main channel response are poorly known
201	(Seminara, 2006). However, as main channel discharge declines, deposition occurs in
202	the main channel below the avulsion point, reducing channel width (Sorrells and
203	Royall, 2014); the main thalweg depth is less affected as long as the main channel
204	discharge remains greater than the cutoff discharge. The relaxation in the system, due
205	to the soft avulsion, results in the main meander r/B increasing as B adjusts more
206	readily than r , which sustains potential for bank erosion downstream of the avulsion
207	as flow is increasingly confined by channel narrowing through time (Coleman, 1969).
208	Thus, soft avulsion may assist a channel in maintaining its meandering habit and so
209	delay a catastrophic reduction in sinuosity. Notwithstanding the relaxation due to B, r
210	also increased in three of the meanders, preventing or delaying avulsion (Fig. 3).
211	Meander R1, influenced by Farakka Barrage backwater, cycles from
212	anastomosed-braided to a single-channel braided pattern (Fig. 4). This pattern differs
213	from those of R2–R4, which cycle from avulsive-anastomosed to a sinuous single-
214	channel pattern, as is typical of wandering rivers. Thus, the imposition of the barrage,
215	with consequent accelerated upstream aggradation and reductions in slope and
216	channel depth, but broadening of the channel, caused a shift from a wandering to a
217	braided pattern, as indexed by the values of ε . Thus, our analysis indicates that rapid
218	aggradation in a wandering river (R1) leads to braiding (viz. Carson, 1984, his
219	wandering type II). Moreover, the wandering planform is sustainable through time,
220	with three meanders (R2–R4) adjusting similarly through time from meandering to a
221	straighter main channel planform by the development of bend cutoffs. So, the
222	wandering habit is not necessarily indicative of a channel in short-term transition

223	between single-channel meandering and braiding (Carson, 1984). To date, the
224	reduction in sediment load downstream of the barrage has not changed the channel
225	pattern, but a more stable meandering habit is predicted by Equation 5 (viz. Carson,
226	1984, his wandering type I) and has been observed recently (Hossain et al., 2013).
227	Consequently, a considerable time lag can be associated with any transition. The
228	similar trend in behavior among all four meanders through similar time scales is
229	highly significant in that criticality develops naturally in the meandering system.
230	Clearly, the meanders are affected by the barrage. Nevertheless, the boundary
231	conditions of a critical bend radius relative to channel apex width, the imposed
232	discharge, and the aggradation rate drive the development of cutoffs as indexed by M ,
233	which reduces toward unity as the likelihood of cutoff becomes pronounced. This
234	behavior develops independently of the presence of negligible bank-side vegetation.
235	Thus, although vegetation can constrain planform, its presence is not a prerequisite to
236	enable the wandering river planform to persist. By corollary, the behavior of other
237	wandering rivers could be assessed in terms of cutoff criticality. Although channel
238	behavior is explained by SOC, limitations remain; the detailed cutoff processes and
239	how changes are transmitted beyond the cutoff locale require identification.
240	CONCLUSIONS
241	Low-sinuosity meanders on the Ganges River behaved similarly to each other
242	extending over ~35 yr without downstream translation as sinuosity increased. Two

meanders avulsed toward the end of the period, a third developed a soft avulsion, andthe fourth was close to avulsion.

The critical bend radius-to-width ratio of $\overline{1.29}$ was associated with avulsion. The role of super-elevation was accounted for in the avulsion process, but was small.

247	Rather, as shown for a barrage-effected meander, sinuosity increased once the
248	backwater developed fully and aggradation drove the avulsion process.
249	Self-organized criticality, with a mobility number (M) tracking meander
250	development, showed that the critical transition is defined by $M \approx 1$ when avulsion
251	was imminent (Fig. 4). Channel phase space (Fig. 4) defined by Parker's braiding
252	criterion and <i>M</i> demonstrates that the meander upstream of the barrage adjusted from
253	an anastomosed braided system to a single-thread braided channel. Downstream, the
254	system follows a wandering river trajectory varying through time from a meandering
255	to an avulsive-anastomosed planform and then returns to meandering after \sim 35 yr.
256	ACKNOWLEDGMENTS (?)
257	[[Would you like to acknowledges paper's reviewers (by name) in an
258	Acknowledgments section?]]
259	REFERENCES CITED
260	Bak, P., 1996, How Nature Works: The Science of Self-Organized Criticality: New
261	York, Springer-Verlag, 212 p., doi:10.1007/978-1-4757-5426-1.
262	Begin, Z.B., 1986, Curvature ratio and rate of river bend migration—Update: Journal
263	of Hydraulic Engineering, v. 112, p. 904–908, doi:10.1061/(ASCE)0733-
264	9429(1986)112:10(904).
265	Braudrick, C.A., Dietrich, W.E., Leverich, G.T., and Sklar, L.S., 2009, Experimental
266	evidence for the conditions necessary to sustain meandering in coarse bedded
267	rivers: Proceedings of the National Academy of Sciences of the United States of
268	America, v. 106, p. 16,936–16,941, doi:10.1073/pnas.0909417106.
269	Camazine, S., Deneubourg, JL., Franks, N., Sneyd, J., Theraulaz, G., and Bonabeau,
270	E., 2001, Self-Organization in Biological Systems: Princeton, New Jersey,
271	Princeton University Press.[[Provide total number of pages]]

- 272 Carson, M.A., 1984, Observations on the meandering-braided river transition,
- 273 Canterbury Plains, New Zealand: Part two: New Zealand Geographer, v. 40,
- 274 p. 89–99, doi:10.1111/j.1745-7939.1984.tb01044.x.
- 275 Chang, H.H., 1983, Energy expenditure in curved open channels: Journal of
- 276 Hydraulic Engineering, v. 109, p. 1012–1022, doi:10.1061/(ASCE)0733-
- 277 9429(1983)109:7(1012).
- 278 Church, M., 1983, Pattern of instability in a wandering gravel bed channel, in
- 279 Collinson, J.D., and Lewin, J., eds., Modern and Ancient Fluvial Systems:
- 280 International Association of Sedimentologists Special Publication 6, p. 169–180,
- doi:10.1002/9781444303773.ch13.
- 282 Coleman, J.M., 1969, Brahmaputra River: Channel processes and sedimentation:
- 283 Sedimentary Geology, v. 3, p. 129–239, doi:10.1016/0037-0738(69)90010-4.
- Edmonds, D.A., Hoyal, D.C.J.D., Sheets, B.A., and Slingerland, R.L., 2009,
- 285 Predicting delta avulsions: Implications for coastal wetland restoration: Geology,
- 286 v. 37, p. 759–762, doi:10.1130/G25743A.1.
- 287 Edmonds, D.A., Paola, C., Hoyal, D.C.J.D., and Sheets, B.A., 2011, Quantitative
- 288 metrics that describe river deltas and their channel networks: Journal of
- 289 Geophysical Research, v. 116, F04022, doi:10.1029/2010JF001955.
- 290 Germanoski, D., and Schumm, S.A., 1993, Changes in braided river morphology
- resulting from aggradation and degradation: The Journal of Geology, v. 101,
- 292 p. 451–466, doi:10.1086/648239.
- 293 Hooke, J.M., 2007, Complexity, self-organisation and variation in behaviour in
- 294 meandering rivers: Geomorphology, v. 91, p. 236–258,
- doi:10.1016/j.geomorph.2007.04.021.

- Hossain, M.A., Gan, T.Y., Basar, A., and Baki, M., 2013, Assessing morphological
- 297 changes of the Ganges River using satellite images: Quaternary International,
- v. 304, p. 142–155, doi:10.1016/j.quaint.2013.03.028.
- Howard, A.D., 2009, How to make a meandering river: Proceedings of the National
- 300 Academy of Sciences of the United States of America, v. 106, p. 17,245–17,246,
- 301 doi:10.1073/pnas.0910005106.
- 302 Howard, A.D., and Knutson, T.R., 1984, Sufficient conditions for river meandering:
- 303 A simulation approach: Water Resources Research, v. 20, p. 1659–1667,
- doi:10.1029/WR020i011p01659.
- 305 Jerolmack, D.J., and Mohrig, D., 2007, Conditions for branching in depositional
- 306 rivers: Geology, v. 35, p. 463–466, doi:10.1130/G23308A.1.
- 307 Kasvi, E., Vaaja, M., Kaartinen, H., Kukko, A., Jaakkola, A., Flener, C., Hyyppä, H.,
- 308 Hyyppä, J., and Alho, P., 2015, Sub-bend scale flow-sediment interaction of
- 309 meander bends—A combined approach of field observations, close-range remote
- sensing and computational modelling: Geomorphology, v. 238, p. 119–134,
- doi:10.1016/j.geomorph.2015.01.039.
- 312 Leopold, L.B., Bagnold, R.A., Wolman, M.G., and Brush, L.M., 1960, Flow
- resistance in sinuous or irregular channels: U.S. Geological Survey Professional
- 314 Paper 282-D, p. 111–134.
- 315 Parker, G., 1976, On the cause and characteristic scales of meandering and braiding in
- 316 rivers: Journal of Fluid Mechanics, v. 76, p. 457–480,
- 317 doi:10.1017/S0022112076000748.
- 318 Prokopenko, M., Polani, D., and Ay, N., 2014, On the cross-disciplinary nature of
- 319 guided self-organization, *in* Prokopenko, M., ed., Guided Self-Organization:
- 320 Inception: New York, Springer, p. 3–15, doi:10.1007/978-3-642-53734-9_1.

- 321 Seminara, G., 2006, Meanders: Journal of Fluid Mechanics, v. 554, p. 271–297,
- doi:10.1017/S0022112006008925.
- 323 Sorrells, R.M., and Royall, D., 2014, Channel bifurcation and adjustment on the upper
- 324 Yadkin River, North Carolina (USA): Geomorphology, v. 223, p. 33–44,
- doi:10.1016/j.geomorph.2014.06.020.
- 326 Stølum, H.-H., 1996, River meandering as a self-organization process: Science,
- 327 v. 271, p. 1710–1713, doi:10.1126/science.271.5256.1710.
- 328 Stølum, H.-H., 1998, Planform geometry and dynamics of meandering rivers:
- 329 Geological Society of America Bulletin, v. 110, p. 1485–1498, doi:10.1130/0016-
- 330 7606(1998)110<1485:PGADOM>2.3.CO;2.
- van Dijk, W.M., Schuurman, F., van de Lageweg, W.I., and Kleinhans, M.G., 2014,
- Bifurcation instability and chute cutoff development in meandering gravel-bed
- 333 rivers: Geomorphology, v. 213, p. 277–291,
- doi:10.1016/j.geomorph.2014.01.018.
- 335 Wasson, J., 2003, A sediment budget for the Ganga–Brahmaputra catchment: Current
- 336 Science, v. 84, p. 1041–1047.
- 337 Wu, B., Zheng, S., and Thorne, C.R., 2012, A general framework for using the rate
- law to simulate morphological response to disturbance in the fluvial system:
- Progress in Physical Geography, v. 36, p. 575–597,
- doi:10.1177/0309133312436569.

341 FIGURE CAPTIONS

- 342 Figure 1. Development of Ganges River meanders R1–R4 in A.D. 1972–2011. Inset:
- 343 Location map showing study area.
- 344

345	Figure 2. A: Derivation of maximum channel aggradation rate, Ganges River, India.
346	Triangles show years (Y) of aggradation; squares are years after Farakka Barrage was
347	full. B: Sinuosity of the R1 meander over time. "Full"[[Explain the need for
348	quotation marks (i.e., what is meant by "full")]] channel aggradation accelerates
349	meander sinuosity.[[In the figure, panel B, it is not clear what is meant by "Years
350	of change in Base Level" – do you mean "Year" Qular, as in calendar year)?
351	(Also, "Change" should be capitalized for consistency)]]
352	
353	Figure 3. Mobility number and sinuosity versus year for Ganges River meanders.
354	Circles are mobility number (M) fitted with polynomial functions; squares are
355	sinuosity of main channel; triangles are cutoff sinuosity. Black arrows are cutoff
356	initiation dates; white arrow is date of cutoff failure (see Fig. 1).
357	

- 358 Figure 4. Channel pattern phase space: AB—anastomosed-braided; BS—braided-
- 359 single; AW—wandering; S—sinuous-single. Time trends, labeled with calendar years
- 360 A.D., are shown for Ganges River meanders R1 and R4.