

Carbonation behavior and mechanical performance of low-carbon recycled concrete under different CO₂ pressures

Hao Bao^{a,b,c,*}, Jie Wang^b, Gang Xu^{a,b}, Ruyu Wang^b, Mohamed Saafi^c, Jianqiao Ye^{c,*}
a. Hubei Key Laboratory of Disaster Prevention and Mitigation, China Three Gorges University, Yichang, China
b. College of Civil Engineering & Architecture, China Three Gorges University, Yichang, China
c. Department of Engineering, Lancaster University, Lancaster LA1 4YR, UK
* Correspondence author: baohowhu@163.com (H. Bao), j.ye2@lancaster.ac.uk (J. Ye).

Abstract: The construction industry faces challenges from high cement-related carbon emissions and construction waste. To address carbon reduction, sequestration, and waste utilization, accelerated carbonation tests were conducted on low-carbon recycled concrete (LCRC) to evaluate the effects of recycled aggregate (RA) replacement ratio of natural aggregate (NA), cement replacement ratio by ground granulated blast-furnace slag (GGBS), and external CO₂ pressure on carbonation depth and compressive strength. Microstructural analyses (SEM, EDX, MIP, and TGA-DSC) were conducted to analyze the microstructure, elemental distribution, pore structure, and compositional changes in LCRC before and after carbonation. Results showed that the LCRC carbonation depth increased and the compressive strength decreased with the increase of the RA and cement replacement ratios. The elevated external CO₂ pressure significantly accelerated carbonation, enhancing both carbonation depth and compressive strength. Under supercritical condition (7.5 MPa), the carbonation depth of the LCRC increased by an average of 164% compared to samples at 0.1 MPa, while the average compressive strength was 47% higher than that of the uncarbonated samples. Carbonation converts flocculent C-S-H gel into CaCO₃, refining the pore structure and reducing porosity by 28-34%. Based on the TGA result difference inversion, a method for determining Ca(OH)₂ and C-S-H contents in LCRC was proposed, revealing pre-carbonation contents of 17-37% and 63-83%, respectively. The maximum carbon uptake capacity reached 96.09 kg CO₂/m³ of LCRC, highlighting its potential to support a low-carbon circular concrete industry.

Keywords: Low-carbon recycled concrete; Accelerated carbonation; Carbonation depth; Compressive strength; External CO₂ pressures.

1. Introduction

In recent years, the growing demand for concrete structural maintenance and construction has significantly boosted concrete usage, thereby accelerating the rise in global cement production. CO₂ emissions from cement production account for approximately 8% of global carbon emissions [1, 2]. To reduce the carbon footprint, partially replacing cement with mineral admixtures such as silica fume, fly ash, and GGBS not only improves concrete performance but also conserves energy and reduces emissions [3, 4]. Currently, the construction industry faces both the challenges of managing construction waste in landfills and the decline in NA resources [5, 6]. Recycling waste concrete into RA for new concrete production provides a practical solution to both challenges, yielding significant environmental and economic benefits [7, 8]. Furthermore, carbonation treatment of the concrete not only enhances its mechanical properties [9] but also

40 enables long-term CO₂ sequestration ^[10]. Therefore, carbonation treatment of LCRC at varying RA and
41 cement replacement ratios can yield synergistic benefits, including reduced cement consumption, enhanced
42 RA resource utilization, improved mechanical properties, and carbon sequestration.

43 Compared to the natural aggregate concrete (NAC), recycled aggregate concrete (RAC) shows
44 characteristics like low-density, high-water absorption, and large porosity. By investigating the porosity
45 characteristics at RA interface, Duan et al. ^[11] found that old mortar adhering to the surface of RA is the
46 primary cause of high porosity in the interfacial transition zone (ITZ). Therefore, the partial replacement of
47 NA with RA in concrete inevitably decreases its compressive strength ^[12], splitting tensile strength ^[13] and
48 durability ^[14, 15]. However, this porous ITZ with interconnected pores also allow much higher CO₂ transport
49 rates in RAC compared to in NAC ^[16, 17]. Carbonation treatment can densify RA and promote carbonation of
50 new paste^[18, 19], where CaCO₃ fills the pores and strengthens the microstructure, improving both the
51 mechanical properties and durability of RAC while absorbing CO₂ ^[19, 20]. Silva et al. ^[21] found that the
52 carbonation depth of RAC incorporating 100% RA was 2.15 times that of NAC. Satomi et al. ^[22] investigated
53 the effect of RA replacement ratio on concrete mechanical properties, observing a 21% decrease in
54 compressive strength for RAC incorporating 100% RA compared to NAC. Although compressive strength
55 decreases with increasing RA replacement ratio, carbonation treatment can enhance the compressive strength
56 of RAC ^[23, 24]. Therefore, studies on the carbonation and mechanical properties of LCRC under different RA
57 replacement ratios are of great significance.

58 To mitigate the adverse effects of RAs on concrete mechanical properties, methods such as enhancing
59 the bond strength of cement paste in porous concrete using nanomaterials ^[25] or optimizing the performance
60 of ITZ ^[26] have been explored. However, employing nanomaterials to increase cement paste bond strength is
61 costly, limiting its application. By incorporating mineral admixtures (silica fume, fly ash, and GGBS) or
62 adjusting fine aggregate composition ^[27-30] expands the bonding area between cement paste and aggregate,
63 fills internal voids, thereby enhancing concrete density ^[27]. This approach improves compressive strength ^[31]
64 and durability ^[32] of RAC at the same time reducing cement usage ^[33]. Wang et al. ^[34] achieved an 8% increase
65 in RAC compressive strength by adding 20% fly ash. Qureshi et al. ^[35] demonstrated that incorporating 30%
66 GGBS refined RAC pore size and connectivity through the pozzolanic reaction-generated C-S-H gel,
67 resulting in a 5-12% increase in compressive strength and a 13% reduction in water absorption. Furthermore,
68 with carbonation treatment of concrete containing various mineral admixtures, the carbonation rates
69 accelerate due to reduced alkali reserves, thereby increasing compressive strength ^[36, 37]. Although
70 compressive strength decreases with increasing cement replacement ratios in RAC, carbonation can partially
71 compensate the loss.

72 Accelerated carbonation not only absorbs CO₂ but also enhances compressive strength of RAC.
73 Concrete carbonation involves accelerated carbonation treatment after concrete demolding, with its
74 effectiveness influenced by carbonation temperature ^[38], humidity ^[39], duration ^[40], and CO₂ concentration
75 ^[24]. The influence of CO₂ pressure on accelerated concrete carbonation has drawn significant attention. For
76 instance, Xian et al. ^[41] conducted accelerated carbonation on ordinary concrete under atmospheric pressure
77 and observed a 32% increase in early compressive strength compared to the uncarbonated specimens. Bao et

78 al. [42] studied steel fiber reinforced concrete subject to supercritical carbonation, achieving a maximum
 79 carbonation depth of 32.58 mm within 6 h. Pore volume decreased by up to 32.3% compared to pre-
 80 carbonation levels, while compressive strength increased by 25.1%-42.7% relative to the uncarbonated
 81 specimens. Existing studies have also explored accelerated carbonation of RAC or RA under varying pressure
 82 conditions. For example, Liu et al. [43] found that carbonating RAC incorporating 100% RA at 0.1 MPa (with
 83 CO₂ flow equivalent to 0.3% of cement mass) increased its 28 d compressive strength by 94% compared to
 84 uncarbonated specimens. Zhan et al. [44] conducted carbonation of RAC at 0.4 MPa, observing a 32% increase
 85 in early compressive strength. Ndiaye et al. [45] conducted accelerated carbonation experiments on RA at 2.0
 86 MPa and 5.0 MPa. They observed that increasing CO₂ pressure enhanced its solubility in water, thereby
 87 accelerating the carbonation reaction rate. In summary, the carbonation of cementitious materials is markedly
 88 accelerated under elevated CO₂ pressure, leading to enhanced compressive strength and increased
 89 microstructural compactness. However, comprehensive macro- and micro-scale investigations on the
 90 carbonation behavior and compressive strength of LCRC under different external CO₂ pressures, from
 91 atmospheric to supercritical conditions remain limited.

92 The present study systematically investigates the effects of varying cement replacement ratios, RA
 93 replacement ratios, and external CO₂ pressures on the carbonation depth and compressive strength of LCRC.
 94 A series of microstructural characterization techniques-including scanning electron microscopy (SEM),
 95 energy-dispersive X-ray spectroscopy (EDX), mercury intrusion porosimetry (MIP), and thermogravimetric
 96 analysis (TGA-DSC) were employed to analyze the microstructure, elemental distribution, pore
 97 characteristics, and compositional evolution of LCRC during accelerated carbonation. The contents of C-S-
 98 H and Ca(OH)₂ before carbonation were quantified to assess the carbon sequestration potential of LCRC,
 99 thereby elucidating its carbonation mechanism under different external CO₂ pressures. It simultaneously
 100 demonstrates the dual potential of this approach to synergistically achieve resource recovery from
 101 construction waste and CO₂ sequestration.

102 2. Experimental investigation

103 2.1. Materials

104 2.1.1. Cement

105 Portland cement (P·O 42.5 N) from Anhui Shui'an Construction Group Co., Ltd was used. Its chemical
 106 composition is shown in Table 1, with a density of 2.97 g/cm³.

107 **Table 1** Chemical composition of cement (%).

Components	SiO ₂	Fe ₂ O ₃	Al ₂ O ₃	CaO	K ₂ O	SO ₃	Loss of ignition
By mass (%)	19.27	3.36	3.72	67.11	1.24	4.33	0.97

108 2.1.2. GGBS

109 The GGBS used in this study is grade S95, supplied by Anhui Shui'an Construction Group Co., Ltd. Its
 110 chemical composition is shown in Table 2. Its density is 2.84 g/cm³, and the specific surface area is 424 m²/kg.

111 **Table 2** Chemical composition of GGBS (%).

Components	SiO ₂	Al ₂ O ₃	CaO	MgO	SO ₃	Burn vectors
By mass (%)	33.06	15.04	39.29	9.96	1.9	0.75

112 2.1.3. Fine aggregates

113 The fine aggregate is sourced from natural river sand in the Yichang region. Its fineness modulus is 2.87,
114 with a mud content of 2.7% and an apparent density of 2650 kg/m³. All parameters comply with relevant
115 specifications. The particle size distribution of the river sands is shown in Table 3.

116 **Table 3** Cumulative retained mass of river sands.

Particle size (mm)	4.75	2.36	1.18	0.60	0.30	0.15	<0.15
Cumulative retained mass (%)	0.39	17.65	38.76	60.88	79.79	91.86	99.95

117 2.1.4. Natural and recycled concrete aggregates

118 The NAs were sourced from local quarries in Yichang. The RAs were supplied by Yichang
119 Environmental Protection Building Materials Co., Ltd., produced by crushing the construction waste first
120 using a jaw crusher, and then purifying using air separation and magnetic separation. By mass, the RAs
121 consisted of approximately 96% concrete aggregates and 4% brick aggregates. Before testing, both the NAs
122 and the RAs were sieved by particle size using a vibrating sieve shaker equipped with wire mesh screens of
123 20 mm, 10 mm, and 5 mm apertures placed at the upper, middle, and lower layers, respectively. Aggregates
124 with a particle size range of 5-20 mm were selected for the experiment, while impurities such as highly
125 porous and low-density brick aggregates were removed. The particle size distributions of (5-20 mm) were
126 determined according to GB/T 14685-2022 [46]. The results are summarized in Table 4. To ensure adequate
127 workability of the RAC and mitigate the adverse effects of the high water absorption of the RA on the
128 concrete mix proportion, all the RAs were pre-treated to a saturated surface-dry (SSD) condition [47].
129 Specifically, the RA was immersed in a water tank for 24 h to achieve a uniform and stable moisture content
130 [48]. The aggregate surface was then wiped with a dry cloth until no visible water film remained, achieving
131 the saturated surface dry state [49].

132 **Table 4** Cumulative screen residue of RA.

Particle size (mm)	20	16	10	5
Cumulative screen residue by mass (%)	0.00	21.6	52.9	96.15

133 The physical properties of the NA and RA were tested according to GB/T 14685-2022 [46] and GB/T
134 25177-2010 [50], respectively. For the crushing test, 3000 g of 9.5-19 mm aggregate was subjected to 200 kN
135 pressure loading. The pass rate through a 2.36 mm sieve opening was calculated. The specific test results are
136 summarized in Table 5. The results indicate that the crushing value of the RA meets the specification
137 requirements.

138 **Table 5** Physical properties of coarse aggregates.

Coarse aggregates type	Apparent density (kg/m ³)	Water absorption (%)	Crush value (%)
NAs	2820	3.8	7.27
RAs	1974	18.7	29.3

139 2.2. Mix proportion and preparation

140 To investigate the influence of RA replacement ratios, cement replacement ratios, and external CO₂
141 pressure on the carbonation depth and compressive strength of LCRC, cement replacement ratios of 0%, 10%,
142 20%, and 30%, and RA replacement ratios of 0%, 50%, and 100% were adopted. The external CO₂ pressure

143 was controlled at 0.1, 1.0, 3.0, 5.0, and 7.5 MPa. In this study, NA was replaced by RA on a mass basis,
 144 resulting in an increased aggregate volume fraction due to the lower density of RA. This volumetric effect
 145 was regarded as an inherent feature of RA incorporation and was retained to reflect practical engineering
 146 conditions. The mix proportions of the LCRC are shown in Table 6, with a total of 12 mix designs. To reduce
 147 the effect of material heterogeneity on the carbonation results, for each mix design, 3 specimens were used
 148 to measure the average compressive strength after carbonation, and 3 specimens for the carbonation depth.
 149 Considering five levels of external CO₂ pressure for each mix, a total of 30 specimens were tested. In addition,
 150 for each mix design, 3 specimens were selected for compressive strength testing before carbonation, and 3
 151 specimens were used for drying curve measurements of uncarbonated specimens. Therefore, 36 specimens
 152 were cast for each mix design, resulting in a total of 432 specimens.

153 Due to the limited internal space of the reaction chamber, coupled with the installation of a temperature
 154 probe at the center of the chamber lid, which incorporates a vertical sensor extending to the bottom for
 155 continuous monitoring of the temperature inside the chamber, the LCRC specimens were fabricated as cubes
 156 with a side length of 70.7 mm. This dimension allows the chamber to accommodate up to nine specimens
 157 without obstructing the temperature probe. The same dimension of the concrete cubes were also used in prior
 158 investigations to quantify carbonation depth [51, 52] and to evaluate compressive strength before and after
 159 carbonation [53, 54].

160 Before casting the LCRC specimens, the inner surfaces of the molds were uniformly coated with LF-
 161 50305 water-based release agent. This milky-white emulsion, manufactured by Wenzhou Nuofu Building
 162 Materials Co., Ltd., requires about 30 min to form a stable film. Its primary constituents include sodium
 163 dodecyl benzenesulfonate, among others. The concrete was cast using the mix proportions presented in Table
 164 5. The slump of the fresh concrete was measured according to GB/T 50080-2016 [55] by filling the mold in
 165 layers (25 tamping strokes per layer), vertically removing it, and measuring the height difference between
 166 the mold and the slumped specimens. The specimens were demolded after 24 h. During the curing period,
 167 cementitious materials undergo hydration, and their internal pores are typically filled with a saturated
 168 Ca(OH)₂ solution [56]. To ensure an initial pore water saturation of 100% and maintain a relative humidity of
 169 no less than 95% during subsequent curing, this measure prevents leaching of Ca²⁺ from the cementitious
 170 matrix and facilitates continuous hydration of the cement in a high-humidity environment. The demolded
 171 specimens were immersed in a 0.0118 mol/L saturated Ca(OH)₂ solution and cured at (20±2) °C for 28 d.
 172 This procedure ensured control over the initial pore water saturation of the specimens.

173 **Table 6** Mix proportions and 28 d compressive strength of LCRC cured in saturated Ca(OH)₂ solution.

Groups	W/C	Cement (kg/m ³)	GGBS (kg/m ³)	Fine aggregates (kg/m ³)	Coarse aggregates		Water (kg/m ³)	Water reducing agent (kg/m ³)	Slump values (mm)
					Natural (kg/m ³)	Recycled (kg/m ³)			
C-CR0%-CA0%	0.49	411.00	0.00	750.00	1037.00	0.00	201.00	10.30	155
C-CR10%-CA0%	0.49	369.90	41.10	750.00	1037.00	0.00	201.00	10.30	160
C-CR20%-CA0%	0.49	328.80	82.20	750.00	1037.00	0.00	201.00	10.29	160
C-CR30%-CA0%	0.49	287.70	123.30	750.00	1037.00	0.00	201.00	10.29	170
C-CR0%-CA50%	0.49	411.00	0.00	750.00	518.50	518.50	201.00	10.29	160
C-CR10%-CA50%	0.49	369.90	41.10	750.00	518.50	518.50	201.00	10.29	170

Groups	W/C	Cement (kg/m ³)	GGBS (kg/m ³)	Fine aggregates (kg/m ³)	Coarse aggregates		Water (kg/m ³)	Water reducing agent (kg/m ³)	Slump values (mm)
					Natural (kg/m ³)	Recycled (kg/m ³)			
C-CR20%-CA50%	0.49	328.80	82.20	750.00	518.50	518.50	201.00	10.29	165
C-CR30%-CA50%	0.49	287.70	123.3	750.00	518.50	518.50	201.00	10.29	175
C-CR0%-CA100%	0.49	411.00	0.00	750.00	0.00	1037.00	201.00	10.29	170
C-CR10%-CA100%	0.49	369.90	41.10	750.00	0.00	1037.00	201.00	10.29	165
C-CR20%-CA100%	0.49	328.80	82.20	750.00	0.00	1037.00	201.00	10.29	175
C-CR30%-CA100%	0.49	287.70	123.30	750.00	0.00	1037.00	201.00	10.29	185

174 Remarks: C-CR10%-CA50% denotes the LCRC specimens with a cement replacement ratio of 10% and RA replacement ratio
175 of 50%.

176 2.3. Regulation of pore water saturation

177 As a heterogeneous material, a cement-based material exhibits non-uniform pore water distribution [57],
178 which influences its carbonation process [58] and consequently alters the carbonation outcomes. Numerous
179 studies have demonstrated that when the pore water saturation is 0.50 [58, 59], concrete exhibits an increased
180 carbonation rate, a more rapid carbonation process, and a greater carbonation depth. To precisely adjust the
181 pore water saturation to 0.50, the specimens were removed after curing, and their pore water saturation was
182 regulated accordingly. The detailed procedure is described as follows.

183 (1) Three specimens were randomly selected from each mix ratio group listed in Table 5, yielding a
184 total of 36 specimens. After the removal of surface moisture, the saturated mass of each specimen was
185 measured using an electronic balance (Model FD-C60002, Kunshan U-Kovet Electronics Technology Co.,
186 Ltd., China). The average mass for each group, denoted as m_s , was then calculated.

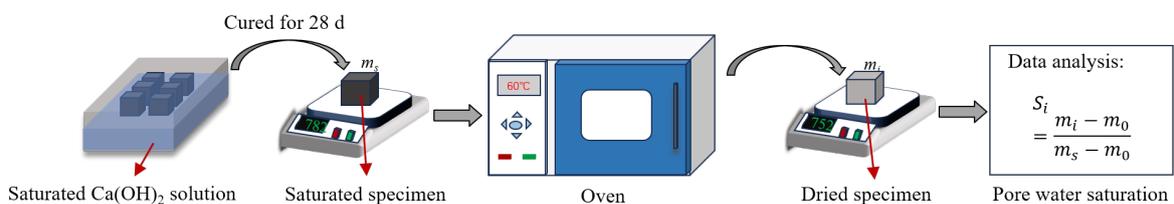
187 (2) The specimens were oven-dried at 60°C. During this process, they were weighed every 8 h for the
188 first 24 h and then every 24 h thereafter, with the average mass of each group recorded as m_i . This drying and
189 weighing cycle was repeated until the mass loss between two successive measurements was less than 0.2 g,
190 at which point the specimens were considered to have reached a completely dry state. The corresponding
191 average mass of each group was denoted as m_0 .

192 (3) From Eq. (1), the pore water saturation of the specimens at each drying interval was calculated and
193 denoted as S_i .

$$S_i = \frac{m_i - m_0}{m_s - m_0} \quad (1)$$

194 where i denotes the drying time (h), m_i represents the mass of the specimen at each drying interval (g), m_0
195 denotes the mass of the specimen after reaching a constant dry state (g), and m_s denotes the mass of the
196 specimen in a fully saturated state (g).

197 The method used to control the pore water saturation of the specimens is illustrated in Fig. 1.



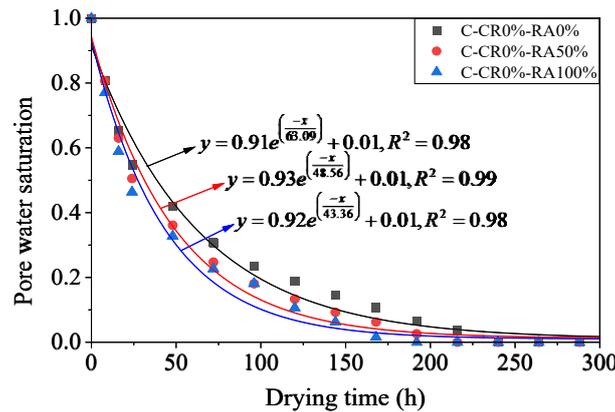
198

199

Fig. 1. Procedure for controlling pore water saturation of specimens.

200 The pore water saturation of specimens at each drying interval was calculated using the above-
 201 mentioned method, and the variation of water saturation against drying time were approximately plotted. It
 202 was found that the overall trend of the pore water saturation for specimens of different mix ratios was
 203 essentially similar. Without loss of generality, the groups with 0% cement replacement and with RA
 204 replacement ratios of 0%, 50%, and 100% were selected as the representative group. The variation of pore
 205 water saturation with drying time for the representative group is shown in Fig. 2. Based on curve fitting,
 206 a mathematical relationship was established and subsequently employed to calculate the time required for the
 207 pore water saturation of the LCRC specimens to reach 0.5.

208 Based on the fitting formula of the drying curve in Fig. 2, determine the drying time for specimens and
 209 dry them according to their respective drying times. After adjusting the pore water saturation to 0.5, all
 210 specimens were pretreated for 30 days in a YH-90B constant temperature/humidity chamber ($20\pm 2^\circ\text{C}$, 50%
 211 relative humidity) to achieve uniform distribution of pore water saturation throughout the specimens [60].



212
 213 **Fig. 2.** Drying curves of LCRC with 0% cement replacement and RA replacement ratios of 0%, 50%, and
 214 100% at 60°C .

215 *2.4. Accelerated carbonation test of LCRC*

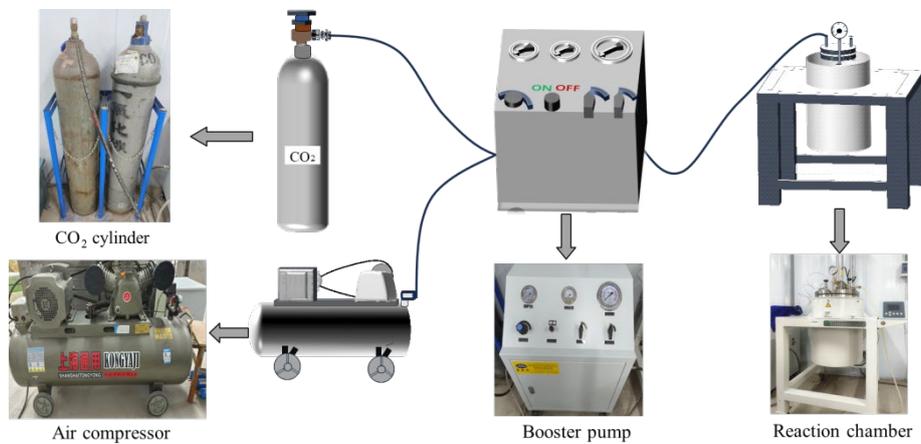
216 To investigate the effects of varying RA and cement replacement ratio on the carbonation depth and
 217 mechanical properties of LCRC under different external CO_2 pressures, a supercritical carbonation apparatus
 218 for LCRC was assembled, as shown by the schematic pressurized carbonation system in Fig. 3. The reaction
 219 chamber (Model SLD20, Beijing Century Langsen Laboratory Instruments Co., Ltd., China) was designed
 220 for a maximum temperature of 200°C and a maximum pressure of 12.5 MPa. The CO_2 pressurization system
 221 comprises a gas booster pump (Model DGS-DGA25-H, Dongguan Saicente Fluid Control Equipment Co.,
 222 Ltd., China), assisted by an air compressor (Model HR230331A, Taizhou Pufeng Electromechanical Co.,
 223 Ltd., China) and a filter (Model 015QPS). The gas booster pump operates within a driving pressure range of
 224 2-8 bars, while the air compressor functions at 0.8 MPa. The CO_2 employed in this experiment was supplied
 225 by Yichang Dexin Practical Gas Co., Ltd., with a purity of $\geq 99.5\%$. The detailed procedures for the
 226 accelerated carbonation test are described as follows.

227 (1) After reaching the target pore water saturation, the specimens were arranged on the material rack
 228 within the reaction chamber. Three specimens were positioned on each layer of the rack, resulting in a total
 229 of three layers. Once all specimens were positioned, the chamber cover was closed and fastened by tightening
 230 the high-strength bolts connecting the cover to the chamber body with a torque wrench.

231 (2) The inlet valve of the reaction chamber was closed, and the outlet valve was opened to connect the
 232 chamber to a vacuum pump (Model OLF750AFV, Shanghai Jaguar Compressor Manufacturing Co., Ltd.,
 233 China). The vacuum pump was then activated until a vacuum of 101.325 kPa was achieved inside the chamber,
 234 at which point the outlet valve was closed and the pump was simultaneously turned off.

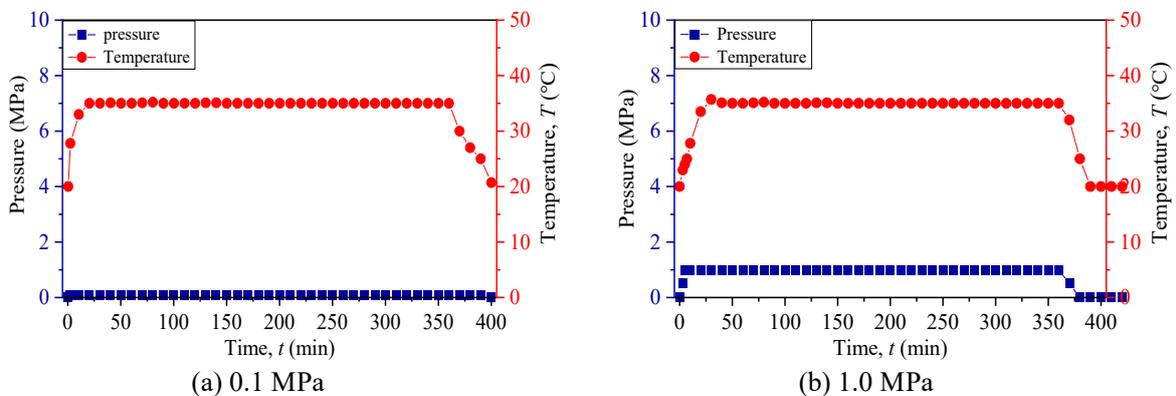
235 (3) The high-pressure valve of the booster pump was opened while the unloading valve was closed.
 236 The CO₂ cylinder valve and the reaction chamber inlet valve were then opened to inject CO₂ into the chamber.
 237 Once the target pressure was reached, both valves were immediately closed. To achieve a pressure above 5.0
 238 MPa, the air compressor was started first, followed by opening the drive gas valve on the booster pump. The
 239 drive gas compressed the CO₂ via the drive piston before injection into the reaction chamber. Upon reaching
 240 the preset pressure, the inlet valve, drive gas valve, and CO₂ cylinder valve were closed sequentially.

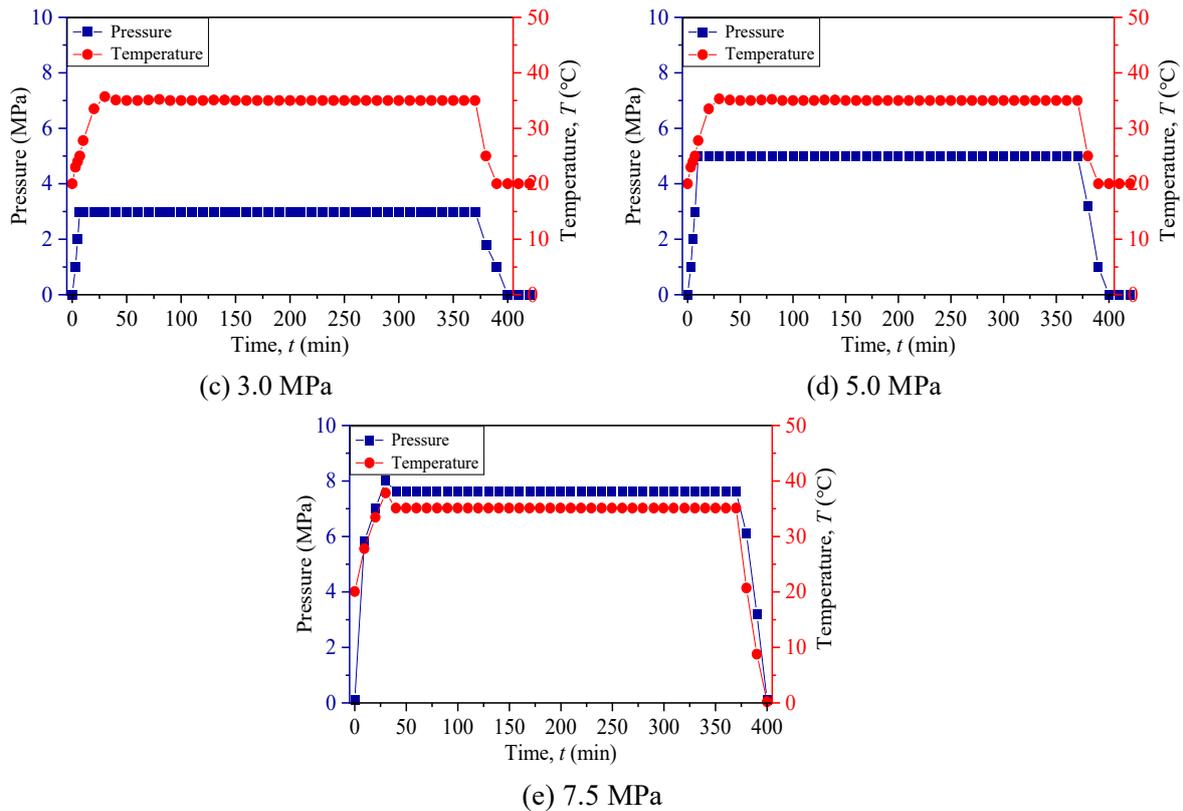
241 (4) The target CO₂ pressures within the reaction chamber were set at 0.1, 1.0, 3.0, 5.0, and 7.5 MPa.
 242 At each of the target pressures, during the pressurization and carbonation stages, the specimens were
 243 maintained at the designed pressure for 6 h, with the temperature and pressure in the reaction chamber being
 244 recorded at 10 min intervals. The temperature and pressure curves for representative specimens during the
 245 carbonation are shown in Fig. 4.



246
 247

Fig. 3. Schematic diagram of the accelerated carbonation system.





248 **Fig. 4.** Temperature and pressure variations of C-CR0%-RA0% specimens over time.

249 2.5. Carbonation depth measurement of LCRC

250 The specimens were taken out and tested for carbonation depth after carbonation reaction. Fig. 5 shows
 251 the testing process. Each sample was first cut in half using a cutting machine, and any residual powder
 252 was removed with a rubber air blower. The carbonation depth was then measured using the phenolphthalein
 253 alcohol indicator method. In accordance with the carbonation test method specified in the standard GB/T
 254 50082-2024 [61], a 1% phenolphthalein solution was prepared and sprayed onto the concrete surface. The
 255 uncarbonated areas turned purple, while the fully carbonated regions remained colorless. Measurements were
 256 taken at 10 mm intervals along the surface using a vernier caliper. The carbonation depth was calculated
 257 using Eq. (2).

$$d = \frac{1}{n} \sum_{i=1}^n d_i \quad (2)$$

258 where d is the average carbonation depth at a curing age of 28 d (mm); d_i is the carbonation depth at each
 259 measurement point (mm); and n is the number of measurement points.

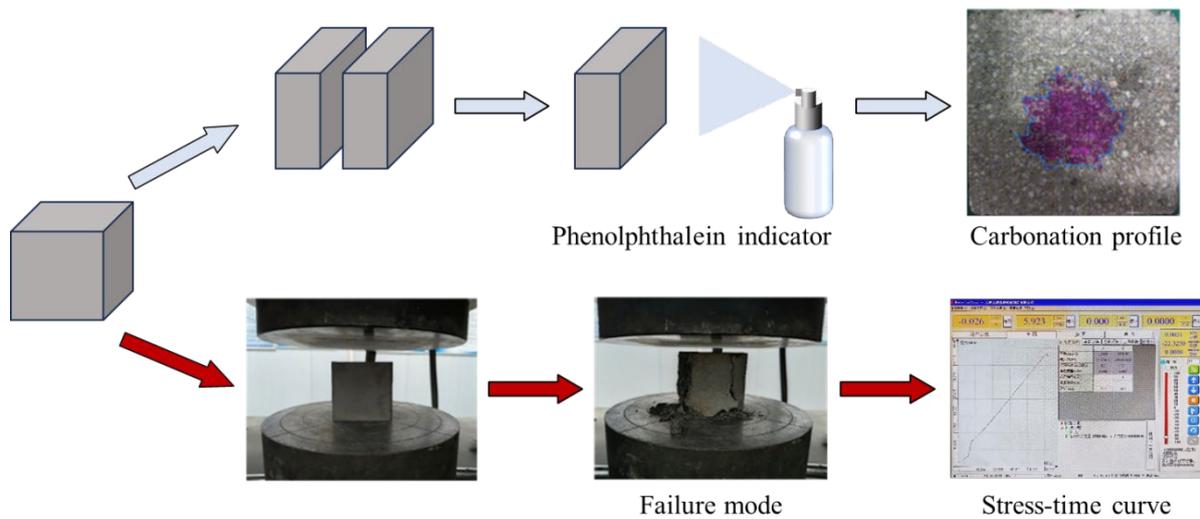
260 2.6. Compressive strength test of LCRC

261 After the carbonation test, some of the specimens were taken out for compressive strength testing, which
 262 was conducted in accordance with GB/T 50081-2019 [62]. The tests were performed using a microcomputer-
 263 controlled electro-hydraulic servo universal testing machine (Model WAW-2000, Shanghai Sansi Zongheng
 264 Machinery Manufacturing Co., Ltd., China) with a maximum load capacity of 2000 kN, applying the load at
 265 a rate of 3 kN/s. The compressive strength was calculated by Eq. (3), while the loads (including the ultimate

266 load) and the corresponding displacements were automatically recorded by the microcomputer system during
267 loading. The compressive strength testing procedure is illustrated in Fig. 5.

$$f_c = \frac{F_{max}}{A} \quad (3)$$

268 In Eq. (3), f_c is the compressive strength of the specimen (MPa), F_{max} is the failure load (N), and A is the
269 bearing area of the specimen (mm^2). For each group, the compressive strength was taken as the arithmetic
270 mean of the three measured specimens, with an accuracy of 0.1 MPa.



271

272

Fig. 5. Schematic diagram of carbonation depth and compressive strength test of LCRC.

273 2.7. Characterization of microstructural and compositional evolution

274 2.7.1. SEM observations

275 To investigate the microstructural characteristics of the LCRC before and after carbonation, both
276 uncarbonated and carbonated samples were ground, and particles measuring 2-3 mm were selected.
277 Following cleaning, the samples were oven-dried at 60°C for 24 h. A thin gold (Au) coating was then applied
278 using a JFC-1600 sputtering coater. The crystalline morphology and pore structure of the samples, before and
279 after carbonation under different CO₂ pressures, were examined using a SEM (Model JSM-7500F, JEOL Ltd.,
280 Japan) operated at an accelerating voltage of 20 kV and a magnification of 20000×. Furthermore, elemental
281 composition analysis was performed using a X-ray fluorescence analyzer (Model EDX-7000, Shimadzu
282 Corporation, Japan).

283 2.7.2. MIP measurement

284 To evaluate the effects of supercritical carbonation on the pore structure of concrete with different
285 cement replacement ratios, samples were collected from both the carbonated and uncarbonated zones and
286 subsequently pulverized. Particles with sizes ranging from 3 to 5 mm were selected and immersed in
287 anhydrous ethanol for 48 h to terminate hydration. The samples were then dried at 85°C, cooled to room
288 temperature, and sealed to prevent further carbonation. The pore structure of the prepared specimens was
289 characterized using a porosimeter (Model AutoPore IV 9500, Mc Instruments, Inc., USA). In this study,
290 porosity refers to the total intrusion porosity obtained from MIP measurements. It is defined as the cumulative

291 volume of mercury intruded at the maximum applied pressure, expressed as a percentage of the specimen's
 292 bulk volume. The measurement covers a pore diameter range from approximately 357 μm down to 3 nm.

293 2.7.3. TGA-DSC measurement

294 To investigate the variations in CaCO_3 and Ca(OH)_2 contents in the carbonated and uncarbonated zones
 295 of the LCRC under different CO_2 pressures, samples from the two zones were crushed and ground into
 296 powder. TGA-DSC was performed using a simultaneous thermal analyzer (Model STA449F5, Netzsch,
 297 Germany). Prior to the testing, the samples were oven-dried at 40°C for 24 h, ground into powder, and sieved
 298 through an 80 μm mesh. During the test, nitrogen was used as purge gas, and the specimens were heated to
 299 1000°C at a constant heating rate of $10^\circ\text{C}/\text{min}$, while the mass change of the samples was continuously
 300 monitored.

301 2.8. Quantification of C-S-H and Ca(OH)_2 contents from TGA-DSC results

302 This study employs an analytical method using supercritical carbonation to quantitatively determine the
 303 Ca(OH)_2 and C-S-H contents in concrete prior to carbonation. The method calculates the mass of generated
 304 CaCO_3 and the residual Ca(OH)_2 mass from the thermogravimetric analysis of fully carbonated specimens.
 305 Combined with the initial Ca(OH)_2 content in the uncarbonated specimens, this determines the mass of
 306 Ca(OH)_2 participating in the reaction. In ordinary Portland cement, other hydration products (such as calcium
 307 aluminate hydrates) are present in low concentrations, and the carbonation of un-hydrated components is
 308 negligible [63]. Therefore, it can be assumed that the carbonation reaction originates solely from the
 309 participation of Ca(OH)_2 and C-S-H. Based on this assumption, the C-S-H content involved in the
 310 carbonation reaction can be further calculated by combining the total CaCO_3 formation with the mass of
 311 Ca(OH)_2 participating in the reaction. The specific calculation formula is as follows [60]:

$$m_{CC} = (D_{800} - D_{600}) \times m \quad (4)$$

$$m_{CH} = (D_{550} - D_{400}) \times m \quad (5)$$

$$m_{CH \rightarrow CC} = \frac{m_{CH}}{M_{CH}} \times M_{CC} \quad (6)$$

$$m_{C-S-H} = \frac{m_{CC} - m_{CH \rightarrow CC}}{3M_{CC}} M_{C-S-H} \quad (7)$$

$$P_{CH} = \frac{m_{CH}}{m_{CH} + m_{C-S-H}} \quad (8)$$

$$P_{C-S-H} = 1 - P_{CH} \quad (9)$$

312 where D_{800} represents the mass loss at 800°C , determined by the TGA-DSC test. m is the mass of the powder
 313 sample used in the TGA-DSC test. m_{CC} refers to the total mass of CaCO_3 produced by the carbonation
 314 reaction, while m_{CH} denotes the mass of Ca(OH)_2 involved in the carbonation reaction. $m_{CH \rightarrow CC}$ represents
 315 the mass of CaCO_3 derived from Ca(OH)_2 . The molar mass of Ca(OH)_2 (M_{CH}) is 74 g/mol, and the molar
 316 mass of CaCO_3 (M_{CC}) is 100 g/mol. Additionally, m_{C-S-H} signifies the mass of C-S-H involved in the
 317 carbonation reaction, with its molar mass (M_{C-S-H}) being 342 g/mol. The percentage content of Ca(OH)_2 and

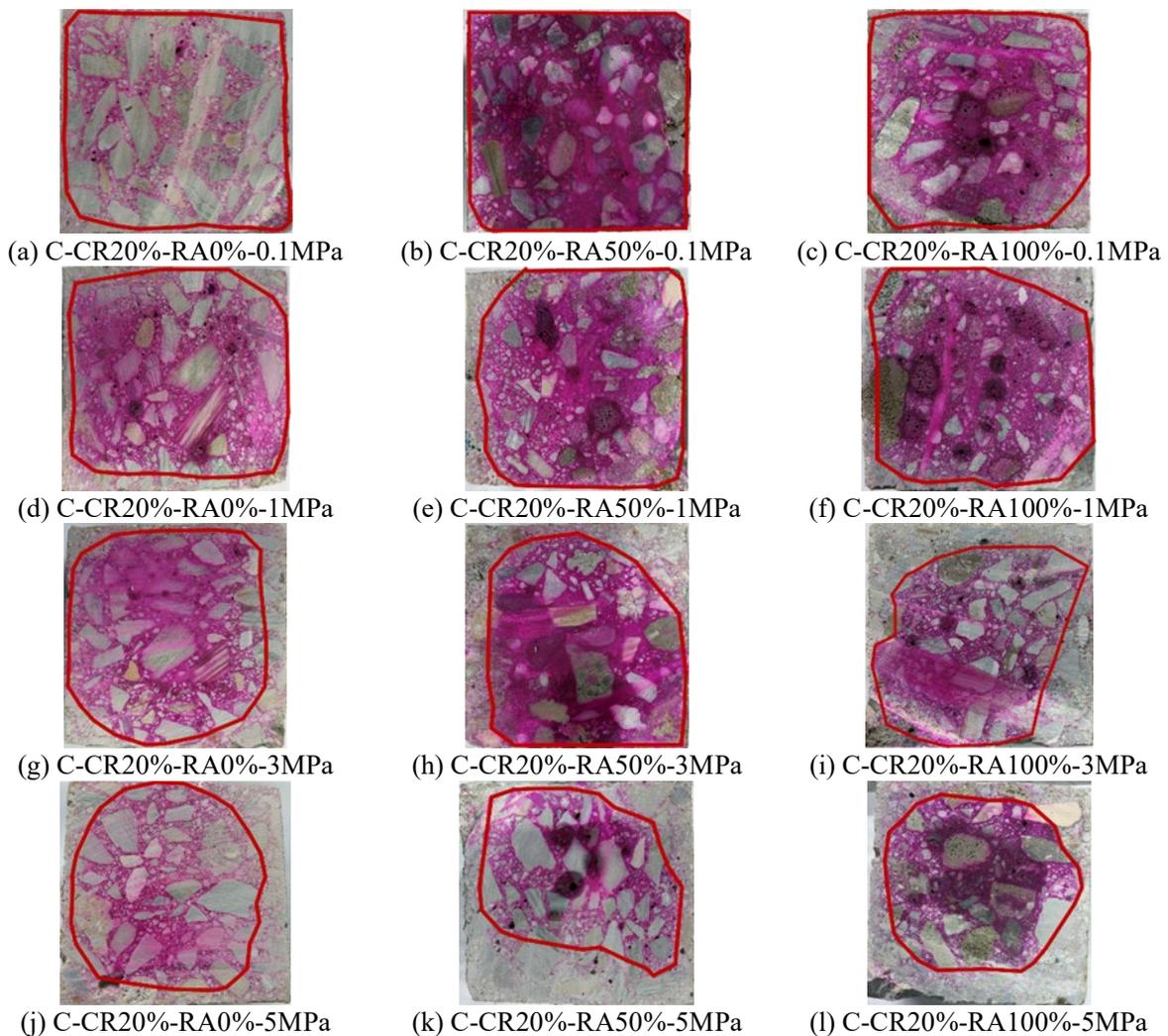
318 C-S-H are denoted as P_{CH} and P_{C-S-H} , respectively.

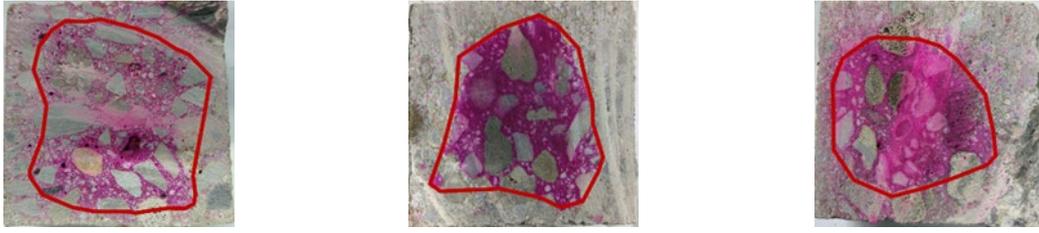
319 3. Results and discussion

320 3.1. Pattern of LCRC carbonation depth variation

321 3.1.1. Effect of RA replacement ratio

322 To evaluate the effect of RA replacement ratio on the carbonation depth of LCRC, specimens were
323 selected with RA replacement ratios of 0%, 50%, and 100%, and cement replacement ratios of 0%, 10%,
324 20%, and 30%. Following the procedure described in Section 2.3, the pore water saturation of these
325 specimens was adjusted to 0.50. Accelerated carbonation tests were then conducted under different pressures
326 according to the method outlined in Section 2.4. The carbonation depth was measured based on the approach
327 specified in Section 2.5. Without loss of generality, the results for the 20% cement replacement LCRC are
328 presented in Fig. 6. After spraying with phenolphthalein solution, the concrete surface showed distinct zones:
329 the colorless carbonated areas and the dark purple-red uncarbonated areas. The carbonation depth of all
330 experimental groups was calculated using Eq. (2), and the processed results are shown in Fig. 7.





(m) C-CR20%-RA0%-7.5MPa (n) C-CR20%-RA50%-7.5MPa (o) C-CR20%-RA100%-7.5MPa

Fig. 6. Carbonation morphology of LCRC under different RA replacement ratios.

331

332

333

334

335

336

As shown in Fig. 6, under the identical external CO₂ pressure and cement replacement ratio, the colorless carbonated area indicated by the phenolphthalein solution significantly expanded as the RA replacement ratio increased from 0% to 100%, indicating a clear increase in carbonation depth. Under different RA replacement ratios, shown in Fig. 7, the carbonation depth exhibited an increasing trend with higher RA content across all external CO₂ pressure levels.

337

338

339

340

341

342

343

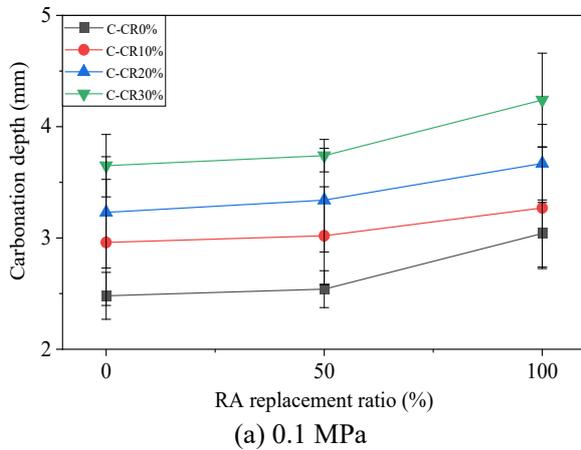
344

345

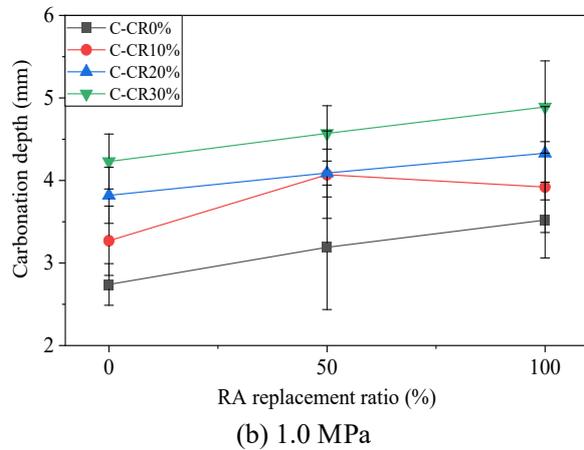
346

347

At an external CO₂ pressure of 0.1 MPa (Fig. 7a), the carbonation depth gradually increased with a 50% RA replacement ratio but rose significantly when the replacement ratio reached 100%. Further analysis of the test results under different pressure conditions (Figs. 7b-e) revealed that, at identical external CO₂ pressure and cement replacement ratio, the average carbonation depth of the LCRC with 50% and 100% RA replacement ratios increased by 13% and 26%, respectively, compared to the LCRC with a RA replacement ratio of 0%. This can be attributed to the higher water absorption (18.7%) and lower apparent density (1974 kg/m³) of the RA (as listed in Table 4), which confirms the higher internal porosity of RAC. This porous nature provides additional diffusion pathways for CO₂, thereby accelerating carbonation. These results are consistent with the previously emphasized conclusion that RA porosity accelerates carbonation [21, 64], confirmed that even in LCRC with GGBS admixture, the permeability of RA remains the dominant factor promoting deeper CO₂ penetration.



(a) 0.1 MPa



(b) 1.0 MPa

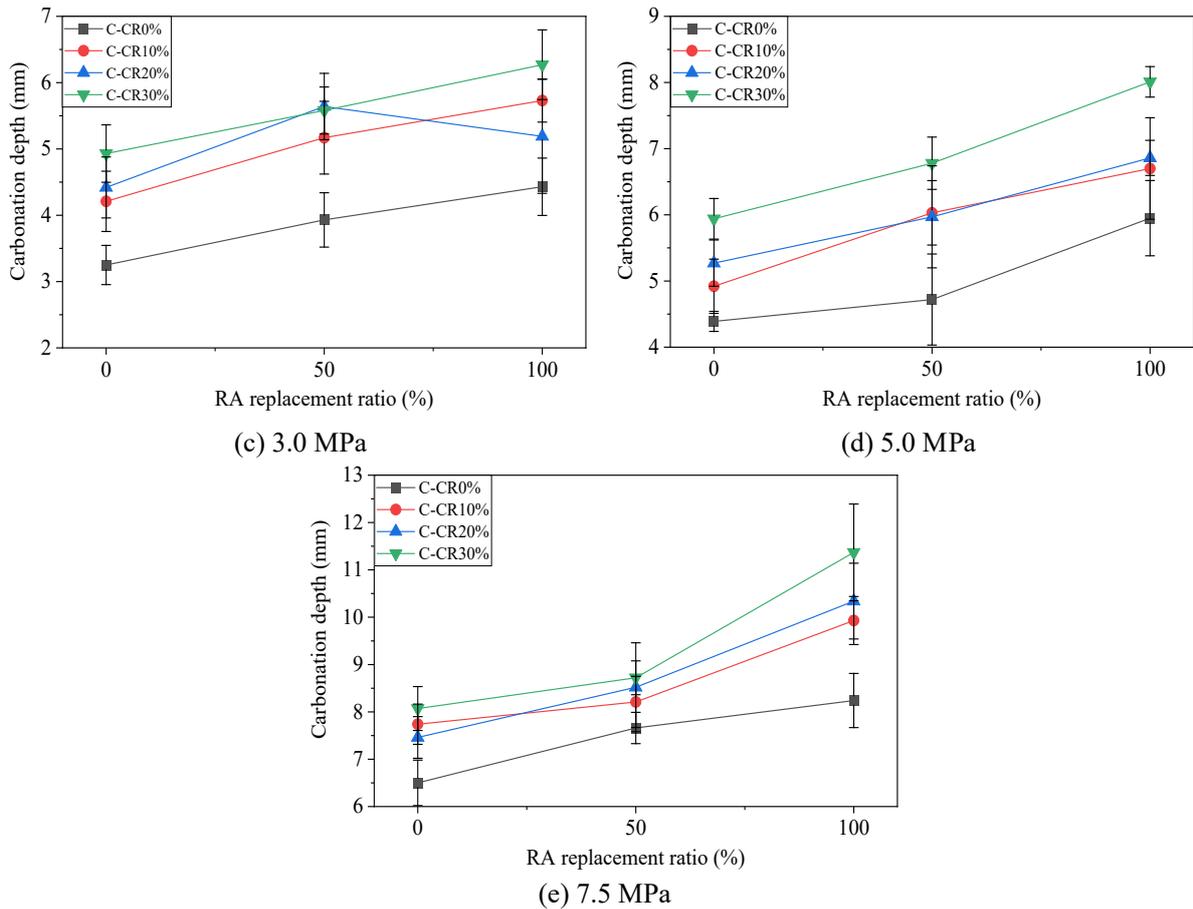
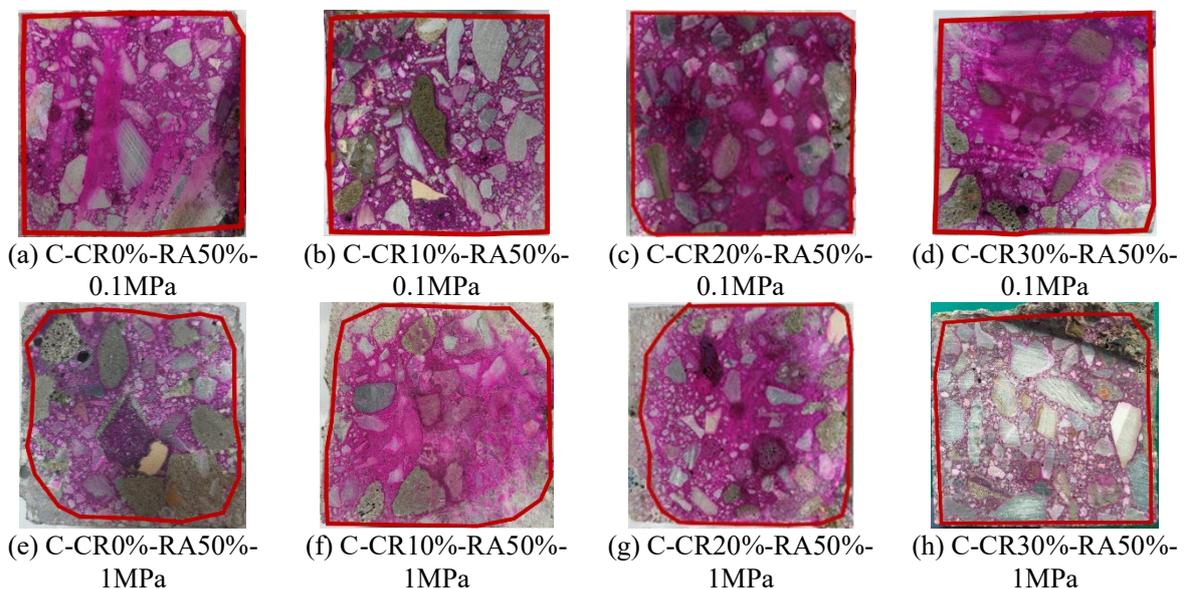


Fig. 7. Carbonation depth of LCRC with different RA replacement ratios.

348

349 3.1.2. Effect of cement replacement ratio

350 To investigate the effect of different cement replacement ratios on the carbonation depth of LCRC,
 351 specimens with a constant RA replacement ratio of 0%, 50%, and 100%, and cement replacement ratios of
 352 0%, 10%, 20%, and 30% were subjected to carbonation under various CO₂ pressures. Without loss of
 353 generality, the carbonation profiles of 50% RA replacement LCRC are shown in Fig. 8, and the calculated
 354 carbonation depths for all experimental groups are summarized in Fig. 9.



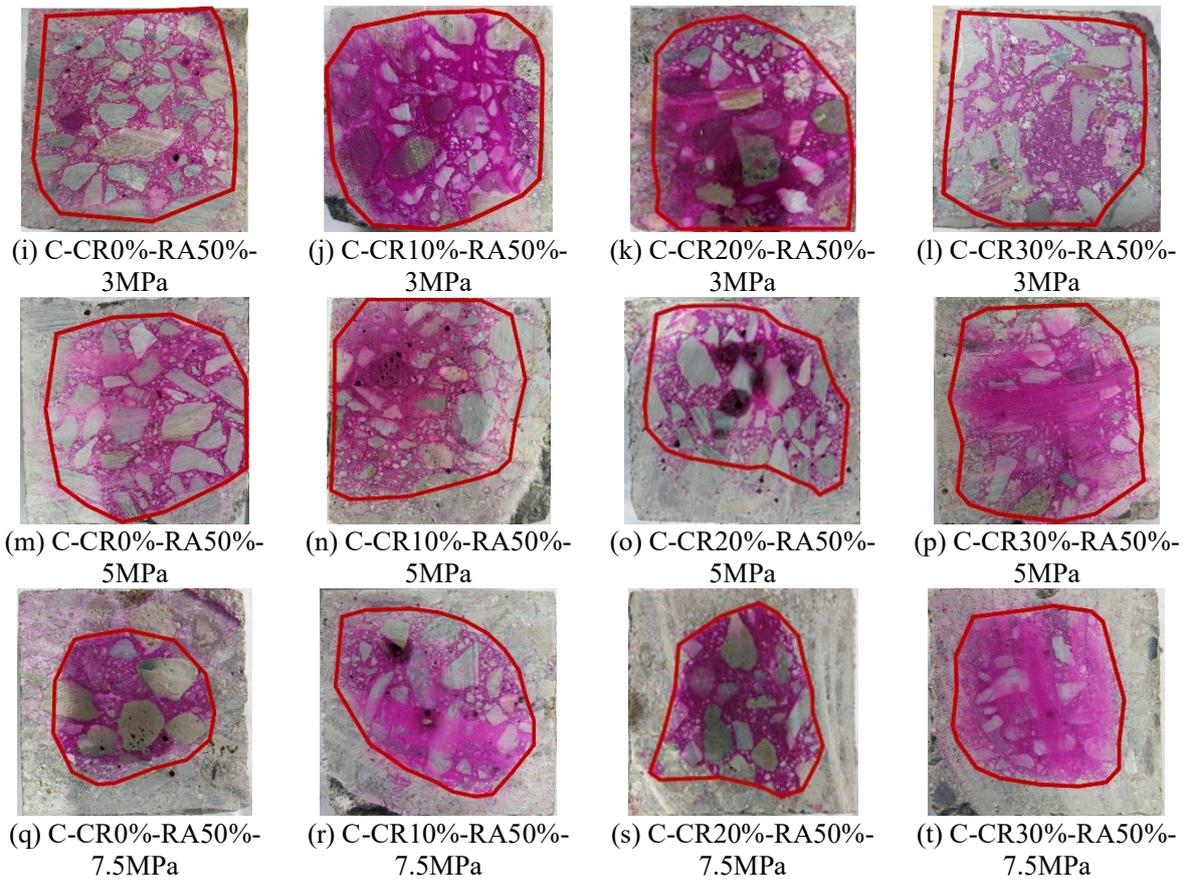


Fig. 8. Carbonation morphology of LCRC under different cement replacement ratios.

355

356

357

358

359

360

361

362

363

364

365

366

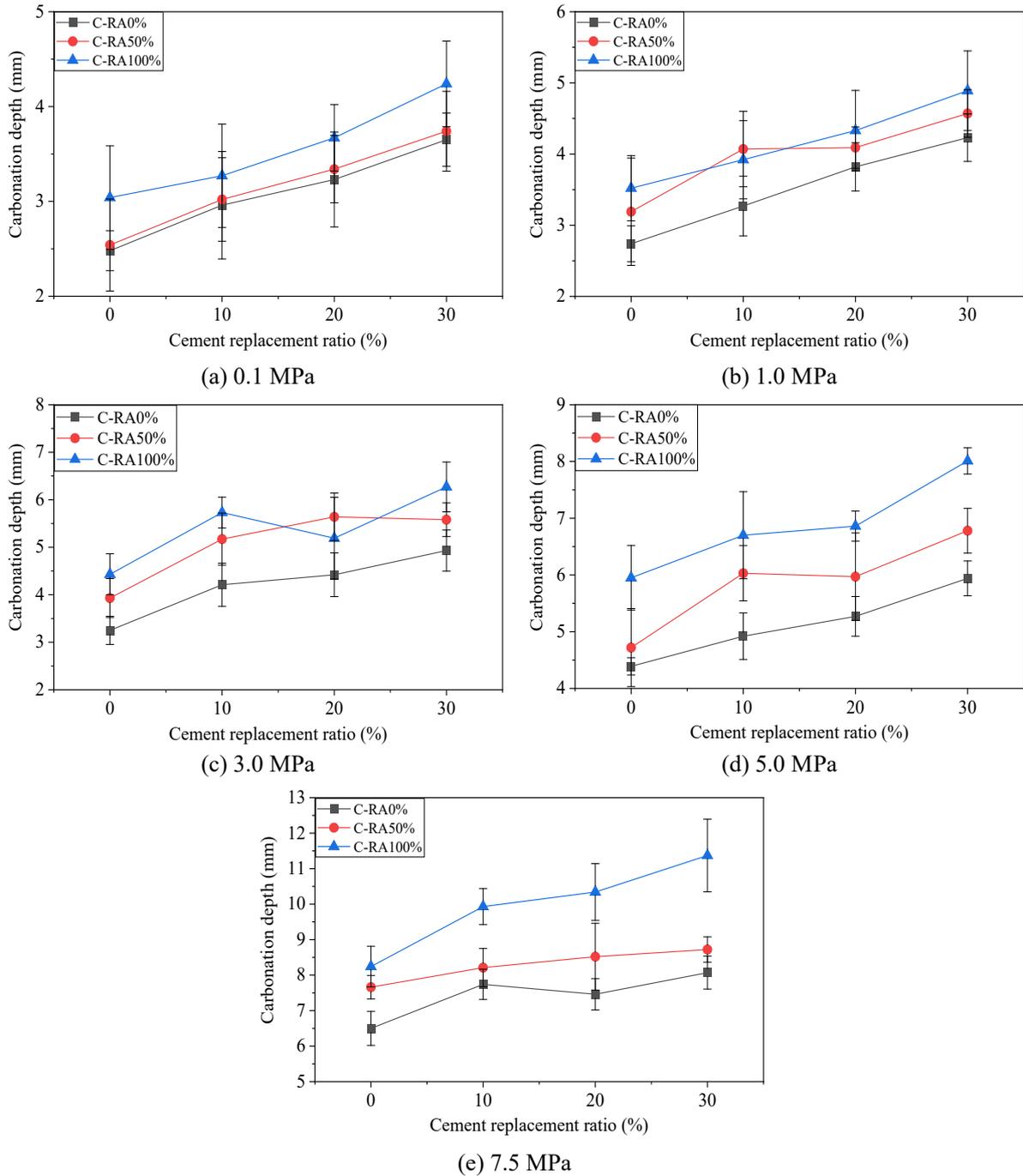
367

368

369

370

[Fig. 8](#) illustrates the influence of the cement replacement ratio on the carbonation morphology of the specimens. As the replacement ratio increases from 0% to 30%, the colorless carbonated region on the cross-section gradually expands, indicating a progressive increase in carbonation depth. In particular, under external CO₂ pressure of 5.0 MPa and 7.5 MPa ([Figs. 8m-t](#)), the carbonation front of the concrete with 30% cement replacement is noticeably deeper and exhibits a more distinct and well-defined boundary compared with the control mix. As shown in [Figs. 9\(a\)-\(e\)](#), under identical external CO₂ pressure and RA replacement ratio, the carbonation depth of LCRC exhibits an increasing trend with rising cement replacement ratios. The carbonation depth of LCRC with 10%, 20%, and 30% cement replacement ratios increased by 20%, 26%, and 40% on average, respectively, compared to the LCRC with 0% cement replacement. This is attributed to the increased cement replacement ratio, reducing cement content, thereby diluting and consuming the Ca(OH)₂ produced during hydration. This leads to a decrease in the alkaline reserve within the LCRC, resulting in increased carbonation depth of LCRC ^[65]. TGA-DSC test results ([Fig. 18](#)) confirm that higher cement replacement ratios reduce the calcium source available for carbonation reactions, allowing CO₂ to penetrate more easily into deeper regions. This indicates that when designing LCRC, a balance must be sought between reducing cement content and maintaining sufficient durability.



371 **Fig. 9.** Carbonation depth of LCRC with different cement replacement ratios.

372 *3.1.3. Effect of external CO₂ pressure*

373 Without loss of generality, Fig. 10 partly shows the carbonation morphology of the LCRC under external
 374 CO₂ pressures of 0.1 MPa, 1.0 MPa, 3.0 MPa, 5.0 MPa, and 7.5 MPa, with RA replacement ratios of 0%,
 375 50%, and 100% and a fixed cement replacement ratio of 20%. The carbonation depth measurements across
 376 all experimental groups are presented in Fig. 11.

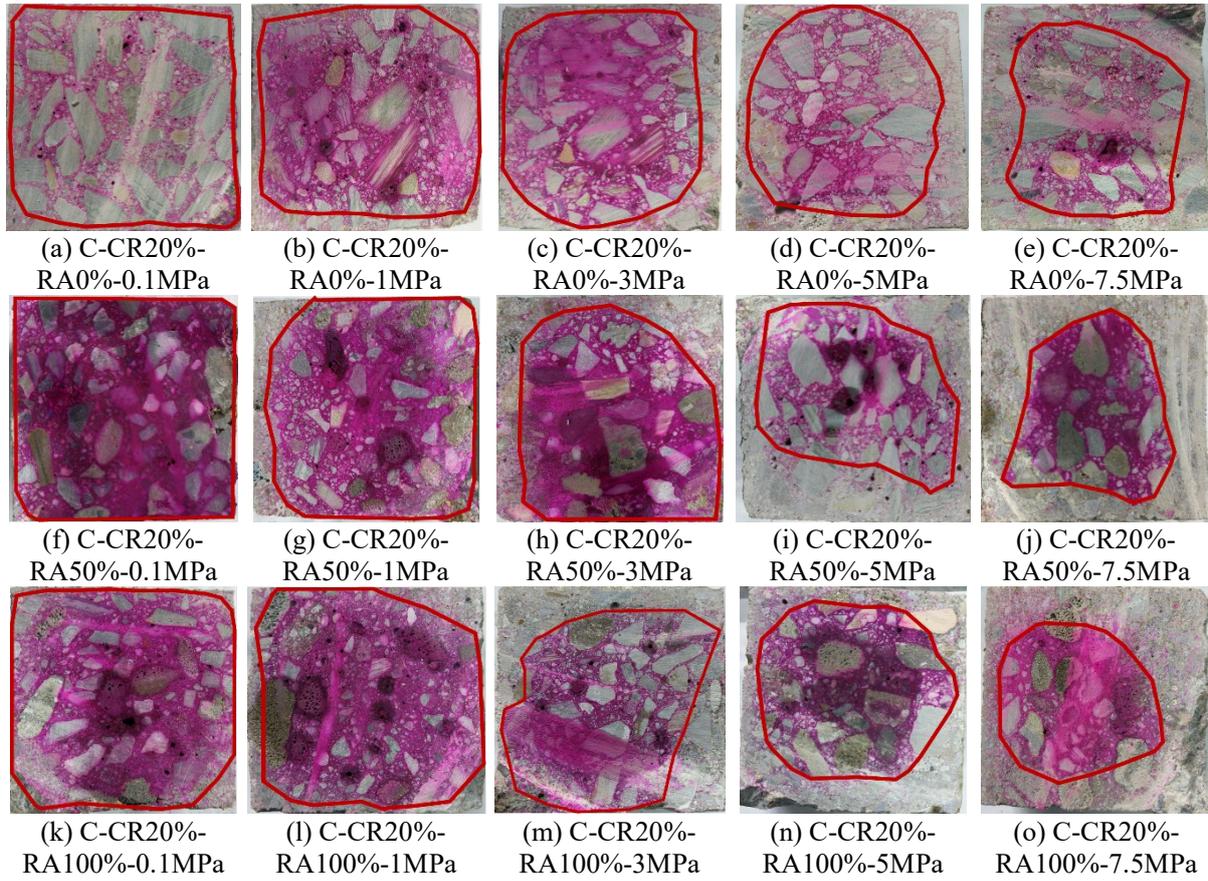


Fig. 10. Carbonation morphology of LCRC under different external CO₂ pressures.

377

378

379

380

381

382

383

384

385

386

387

388

389

390

391

392

393

394

395

396

As the external CO₂ pressure increased from 0.1 MPa to 7.5 MPa, as shown in Figs. 10(k)-(o), the carbonation front of LCRC rapidly migrated from the surface toward the internal zone, with a corresponding significant expansion of the colorless carbonation region. Fig. 11 shows that under identical RA replacement ratios, the carbonation depth of LCRC exhibits nonlinear growth with increasing external CO₂ pressure. At 1.0, 3.0, 5.0, and 7.5 MPa, the average carbonation depth of the LCRC increased by 19%, 50%, 83%, and 164%, respectively, compared with at 0.1 MPa. Within the high-pressure range of 5.0-7.5 MPa, the growth rate of carbonation depth markedly exceeds that observed under lower pressure such as 1.0-3.0 MPa and 3.0-5.0 MPa. For the LCRC with RA replacement ratios of 0% and 50% (Figs. 11a and b), the carbonation depth at 7.5 MPa increased by an average of 144% and 165%, respectively, compared to at 0.1 MPa. When the RA replacement ratio was further increased to 100% (Fig. 11c), the carbonation depth at external CO₂ pressure of 7.5 MPa increased by an average of 181% compared to at 0.1 MPa. This difference primarily stems from the interconnected pore structure introduced by the RA, which provides more efficient diffusion pathways for CO₂ under high pressure, significantly reducing gas transport resistance within the LCRC [66]. Under supercritical CO₂ conditions, pressure gradients become the primary driving force for gas transport. This not only accelerates CO₂ molecule penetration into the LCRC but also substantially enhances its solubility in the pore fluid, thereby accelerating the overall carbonation reaction process [67]. This study quantitatively reveals the enhancement effect of supercritical CO₂ pressure on the carbonation depth of LCRC for the first time, providing a theoretical basis for improving the carbon sequestration efficiency of LCRC by pressurized carbonation technology.

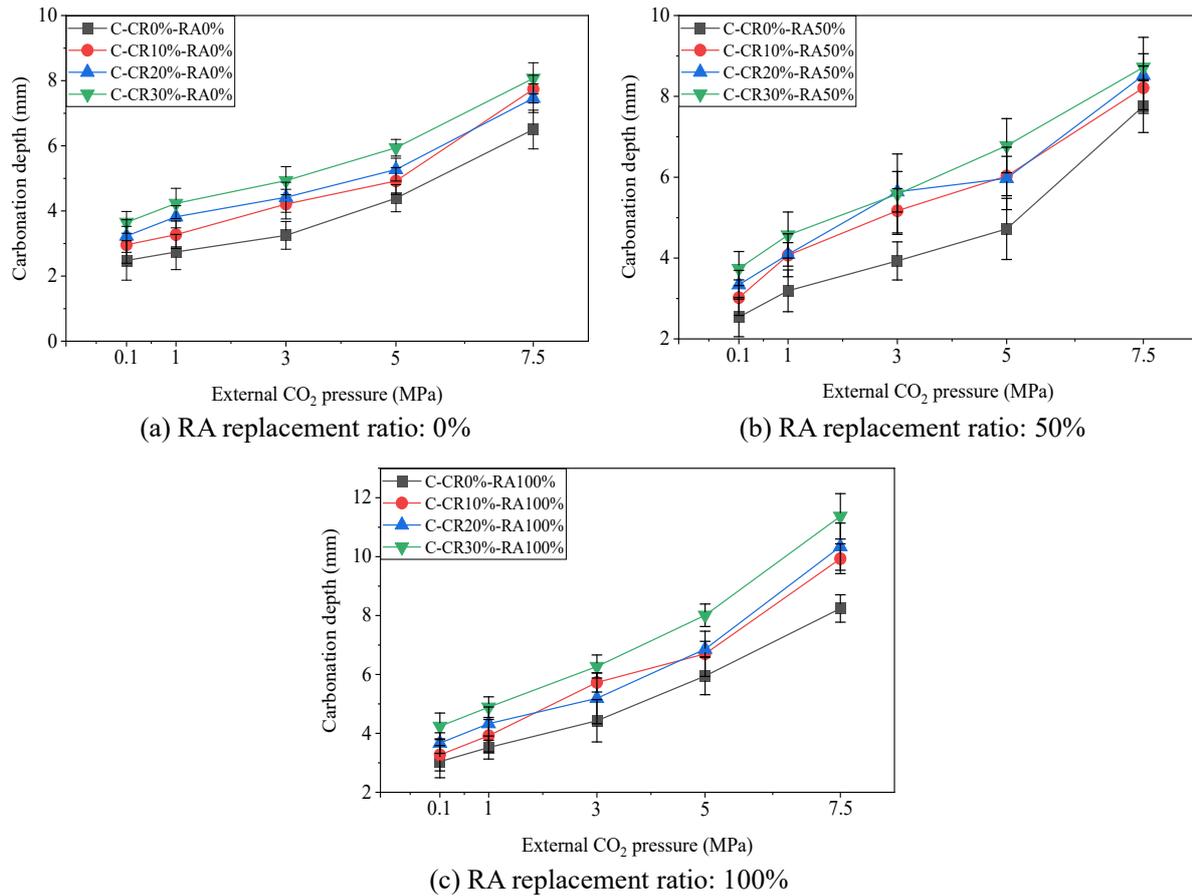


Fig. 11. Carbonation depth of LCRC under different external CO₂ pressure.

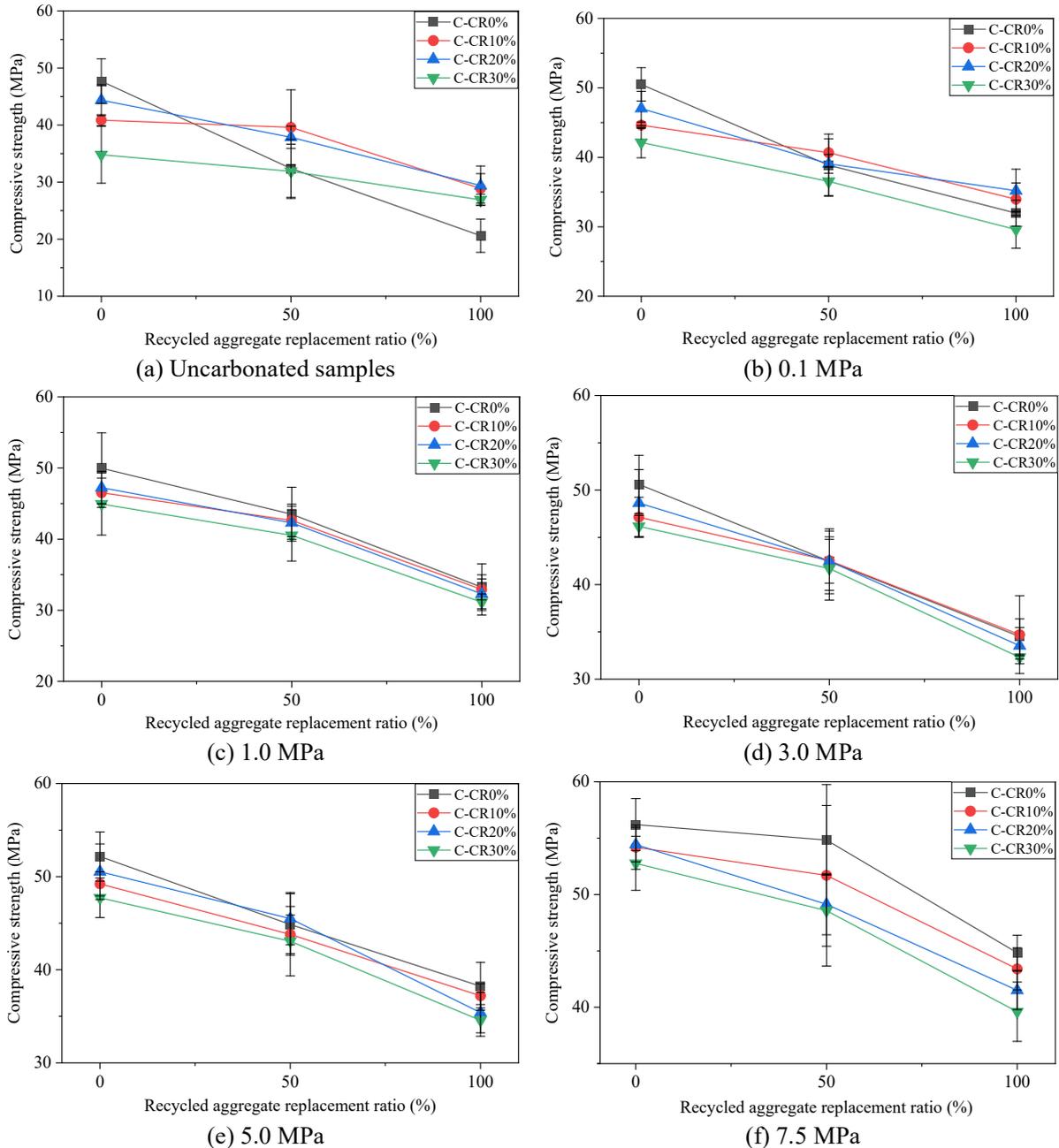
397

398 3.2. Pattern of LCRC compressive strength variation

399 3.2.1. Effect of RA replacement ratio

400 To investigate the effect of different RA replacement ratios on the compressive strength of LCRC,
 401 specimens were selected from both uncarbonated and externally pressurized carbonation conditions.
 402 Following the concrete mechanical property testing method outlined in Section 2.6, the average compressive
 403 strength and standard deviation of the LCRC were calculated and are presented in Fig. 12. Under the
 404 uncarbonated condition and external CO₂ pressures of 0.1 MPa, 1.0 MPa, 3.0 MPa, 5.0 MPa, and 7.5 MPa,
 405 the compressive strength of the LCRC exhibited a decreasing trend with increasing RA replacement ratio. As
 406 shown in Fig. 12(a), under the uncarbonated condition, the compressive strength of the LCRC decreased
 407 significantly with increasing RA replacement ratio. At 100% RA replacement, the compressive strength of
 408 the LCRC was 20.3 MPa, representing a 57% decrease compared to the strength of LCRC with 0% RA
 409 replacement (47.7 MPa). As shown in Figs. 12(b)-(f), under different CO₂ pressure conditions, the strength
 410 reduction was particularly pronounced when the RA replacement ratio increased from 50% to 100%.
 411 Compared to the LCRC without RA, the average strength decreases were 12% and 29%, respectively. This
 412 phenomenon is primarily attributed to the low density and high porosity of RA. They introduce more initial
 413 defects and weak zones into the LCRC matrix, reducing its overall density and adversely affecting its
 414 compressive strength [14]. However, this study reveals that the same porous nature of RA that diminishes
 415 initial strength simultaneously creates a microstructure that is more responsive to strength enhancement

416 through carbonation. SEM images (Fig. 15(j)) clearly show that after carbonation (7.5 MPa), dense CaCO_3
 417 formed within the pores of the new paste region surrounding the RA, indicating that the porous structure of
 418 the aggregate promotes carbonation reactions in the surrounding paste. Consequently, while RA incorporation
 419 reduces the compressive strength of LCRC, it also forms a more permeable microstructure that more
 420 effectively enhances strength through carbonation.

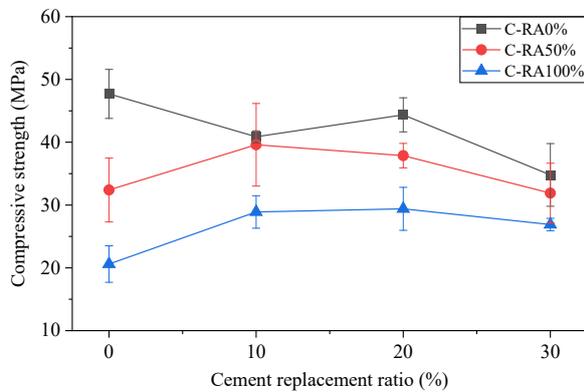


421 **Fig. 12.** Compressive strength of LCRC with different RA replacement ratios.

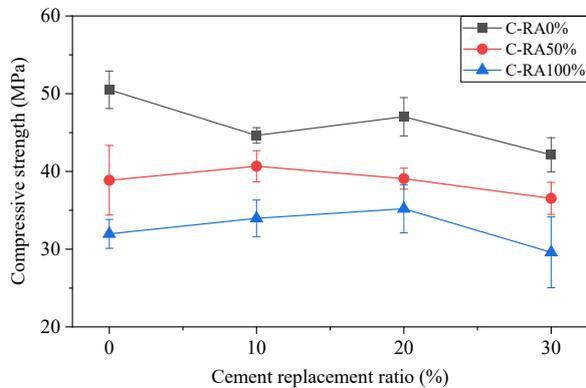
422 *3.2.2. Effect of cement replacement ratio*

423 As shown in Figs. 13(a)-(f), the compressive strength of LCRC exhibits an overall downward trend with
 424 increasing cement replacement ratio, though some fluctuations occur locally. Compared to LCRC with 0%
 425 cement replacement, specimens with 10%, 20%, and 30% cement replacement exhibited average
 426 compressive strength reductions of 3.69%, 3.73%, and 8.34%, respectively. The dilution effect caused by

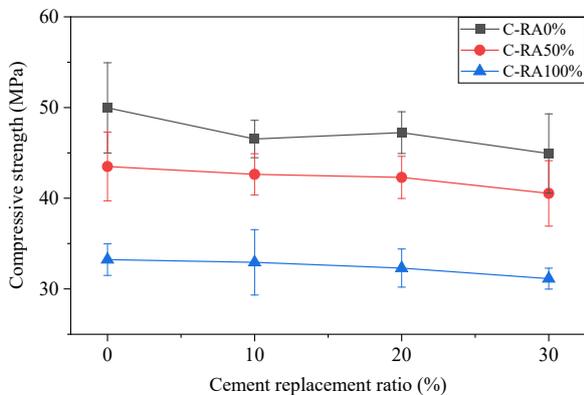
427 reduced cement content slowed the hydration reaction rate of LCRC, thereby weakening its matrix strength
 428 [68]. Under conditions of 0% RA replacement (Figs. 13a-f), the compressive strength of LCRC fluctuated
 429 downward with increasing cement replacement ratio. However, when the RA replacement ratio increased to
 430 50% and 100%, and under non-carbonated conditions with an external CO₂ pressure of 0.1 MPa (Figs. 13a,
 431 b), compressive strength improved with increasing cement replacement ratio. This occurred because the
 432 physical filling effect of GGBS effectively compensated for the initial defects introduced by RA [69]. As
 433 external CO₂ pressure further increased (Figs. 13c-f), the compressive strength of LCRC with RA
 434 replacement ratios of 50% and 100% still exhibited an overall decreasing trend with increasing cement
 435 replacement ratio. TGA-DSC results (Fig. 18) indicate that the absolute content of CaCO₃ formed by
 436 carbonation in LCRC decreases with increasing cement replacement ratio. This is because the increased
 437 cement replacement ratio depleted the alkaline reserve within the LCRC, thereby reducing the strength-
 438 enhancing effect of the carbonation reaction on the LCRC [70]. Notably, the C/Ca atomic ratio significantly
 439 increased in post-carbonation specimens. This indicates that, alongside Ca(OH)₂, C-S-H gel actively
 440 participates in the carbonation reaction and undergoes decalcification, forming amorphous SiO₂ gel. This
 441 phenomenon suggests that in LCRC with higher cement replacement ratio, the degree of carbonation reaction
 442 is greater but the strength is lower.



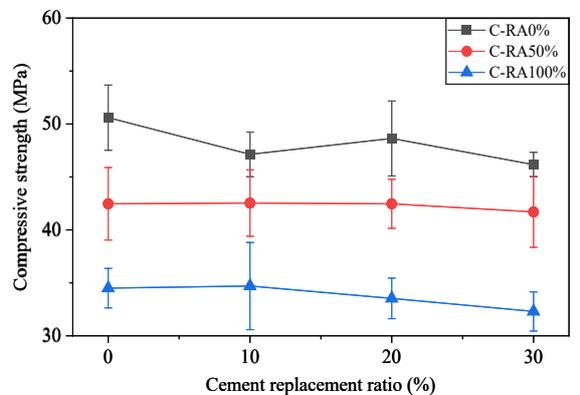
(a) Uncarbonated samples



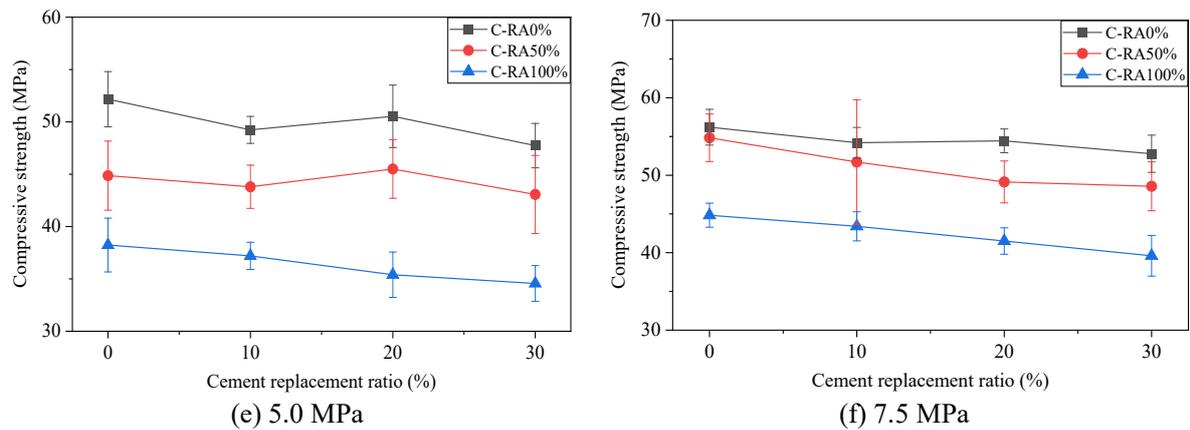
(b) 0.1 MPa



(c) 1.0 MPa



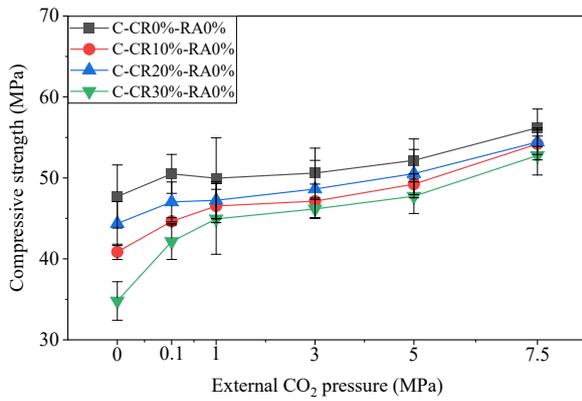
(d) 3.0 MPa



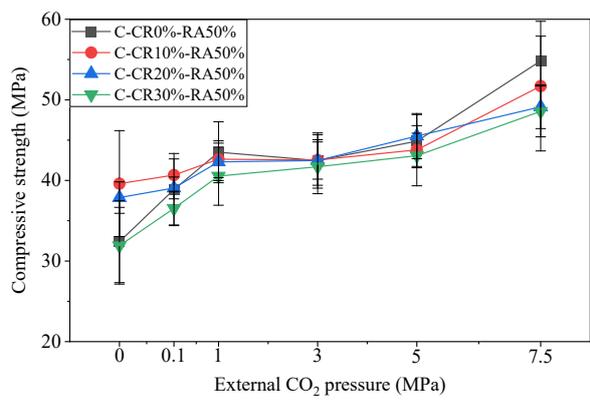
443 **Fig. 13.** Compressive strength of LCRC under different cement replacement ratios.

444 3.2.3. *Effect of external CO₂ pressure*

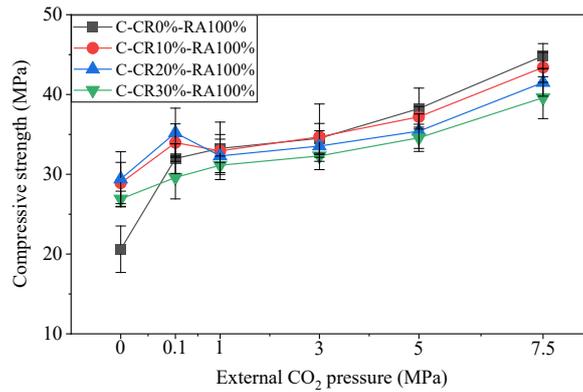
445 As illustrated in Figs. 14(a)-(c), when the replacement ratios of RA and cement are the same, the
 446 compressive strength of the LCRC is greater if it was carbonated under a higher external CO₂ pressure and
 447 reaches its maximum value at 7.5 MPa. Compared with the uncarbonated specimens, the average
 448 compressive strength of the LCRC increased by 15%, 19%, 22%, 29%, and 47% after being carbonated under
 449 external CO₂ pressures of 0.1, 1.0, 3.0, 5.0, and 7.5 MPa, respectively. When the external CO₂ pressure ranged
 450 from 0.1 to 5.0 MPa, the increase in the LCRC compressive strength was relatively gradual. However, the
 451 increase in compressive strength significantly accelerated when pressure rose from 5.0 MPa to 7.5 MPa. This
 452 is because under the supercritical conditions (7.5 MPa), CO₂ exhibits a density close to that of a liquid and a
 453 viscosity close to that of a gas. Consequently, it possesses extremely high diffusion capability and reaction
 454 rate within the LCRC, significantly enhancing the carbonation effect and boosting its compressive strength
 455 [71]. At a 100% RA replacement ratio, the LCRC specimen had a compressive strength of 44.8 MPa after
 456 carbonation at 7.5 MPa, approximately 118% higher than that of its uncarbonated counterpart, which is close
 457 to the compressive strength of the uncarbonated NAC with the same mix proportion. This significant
 458 enhancement can be explained by the evolution mechanisms of specimen porosity and carbonation products.
 459 MIP test results show that the carbonation under external CO₂ pressure of 7.5 MPa reduces the total porosity
 460 by 28.49%-33.83% (Fig. 17). The pore size distribution shifted markedly toward smaller pores, resulting in
 461 a refined pore structure. In contrast to conventional atmospheric carbonation, which typically forms a dense
 462 zone only at the surface and the core performance is relatively weak, supercritical carbonation can promote
 463 the thorough carbonization of Ca(OH)₂ and C-S-H gel deeper inside the specimen and convert it into CaCO₃,
 464 thereby reducing the difference in carbonation degree between the surface and the core of the specimen.



(a) RA replacement ratio: 0%



(b) RA replacement ratio: 50%



(c) RA replacement ratio: 100%

465 **Fig. 14.** Compressive strength of LCRC under different external CO₂ pressures.

466 3.3. *Microstructural and compositional evolution of LCRC under varying CO₂ pressures*

467 3.3.1. *Microstructure evolution*

468 To investigate the effects of cement and RA replacement ratios on the pore structure and crystal
 469 morphology of LCRC, representative specimens were selected, including those with a RA replacement ratio
 470 of 0% and cement replacement ratios of 0%, 10%, 20%, and 30%, as well as a specimen with a RA
 471 replacement ratio of 100% and a cement replacement ratio of 10%. The SEM images of the above groups of
 472 samples under the uncarbonated condition and external CO₂ pressures of 7.5 MPa are shown in Figs. 15(a)-
 473 (j).



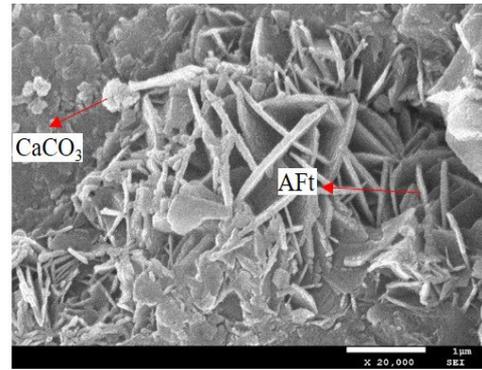
(a) C-CR0%-RA0%-Uncarbonated



(b) C-CR0%-RA0%-7.5MPa



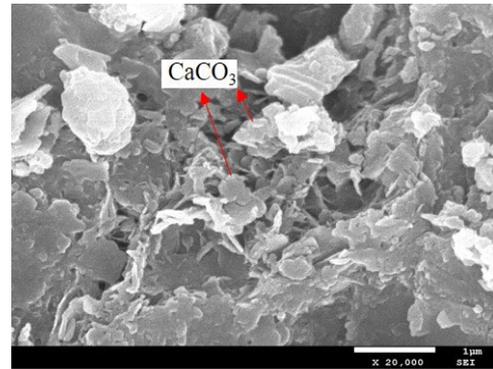
(c) C-CR10%-RA0%-Uncarbonated



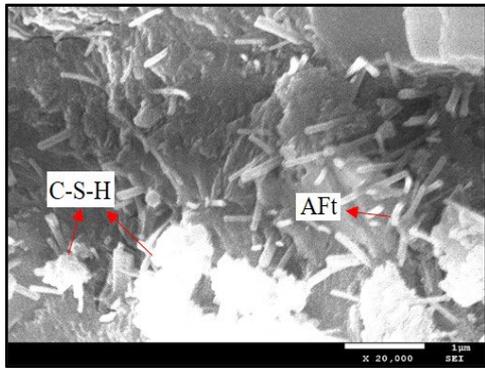
(d) C-CR10%-RA0%-7.5MPa



(e) C-CR20%-RA0%-Uncarbonated



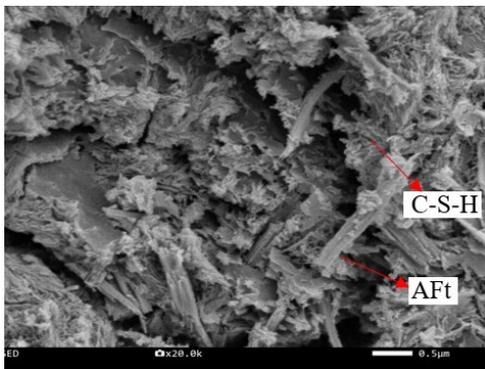
(f) C-CR20%-RA0%-7.5MPa



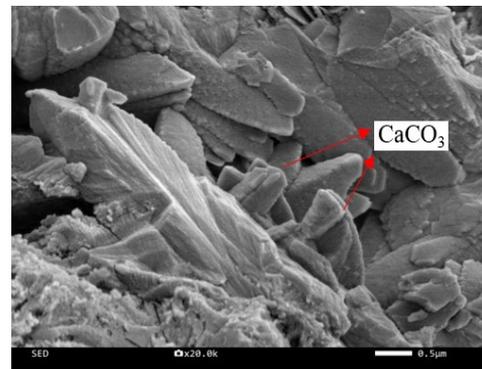
(g) C-CR30%-RA0%-Uncarbonated



(h) C-CR30%-RA0%-7.5MPa



(i) C-CR10%-RA100%-Uncarbonated



(j) C-CR10%-RA100%-7.5MPa

Fig. 15. SEM images of LCRC specimens.

474

475

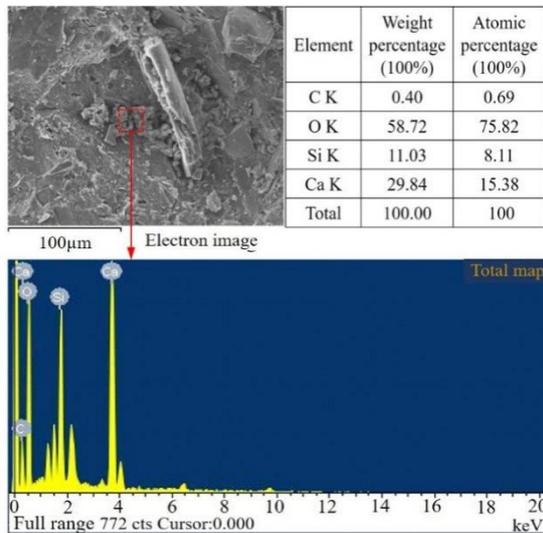
476

477

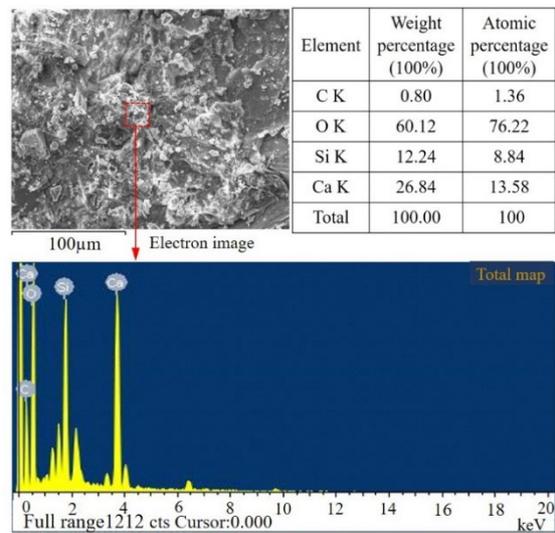
In the uncarbonated state, the hydration products of the LCRC specimens with cement replacement ratios of 0% and 10% mainly comprise lamellar Ca(OH)_2 crystals, flocculent C-S-H gel, and acicular AFt crystals (Fig. 15(a), (c)). As the cement replacement ratio increased to 20% and 30% (Fig. 15(e), (g)), the

478 amount of $\text{Ca}(\text{OH})_2$ generated in the matrix decreased significantly. Only a small amount of needle-like Aft
 479 crystals was visible, and the main hydration products shifted to flocculent and reticular C-S-H gel structures.
 480 This occurs because the reduced cement content diminishes the system's alkaline reserve, limiting $\text{Ca}(\text{OH})_2$
 481 formation. When the RA replacement ratio reaches 100%, the overall morphological characteristics of the
 482 hydration products remain largely unchanged. However, the matrix surrounding the RA exhibits a more
 483 porous microstructure, as shown in Fig. 15(i). After the 7.5 MPa carbonation treatment (Fig. 15(b), (d), (f),
 484 (h)), $\text{Ca}(\text{OH})_2$ and C-S-H hydration products were transformed into lamellar and granular CaCO_3 crystals,
 485 effectively filling pores and significantly enhancing the micro-compactness of the LCRC [72]. In specimens
 486 with a 100% RA replacement ratio, post-carbonation microstructural observations reveal a dense
 487 accumulation of calcium carbonate crystals in the regions surrounding the RA (Fig. 15(j)). This suggests that
 488 the porous structure associated with RA promotes CO_2 transport and reaction, thereby leading to more
 489 pronounced deposition of carbonation products at the aggregate-cement paste interface.

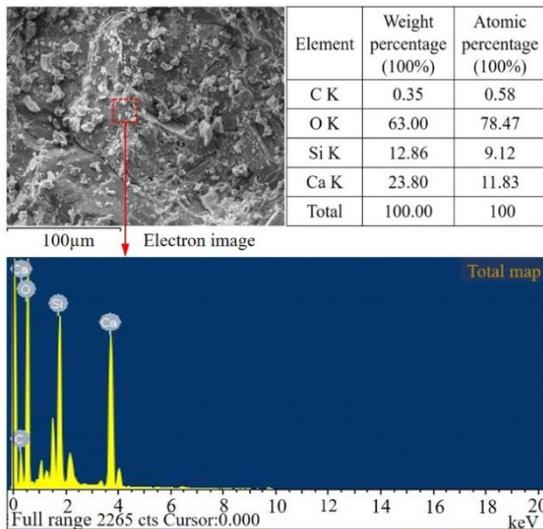
490 To further investigate the degree of carbonation in LCRC under different cement and RA replacement
 491 ratios, this study conducted EDX elemental analysis on samples from both carbonated and uncarbonated
 492 zones. Figs. 16(a)-(e) present the EDX analysis of the LCRC under the uncarbonated condition and external
 493 CO_2 pressures of 7.5 MPa.



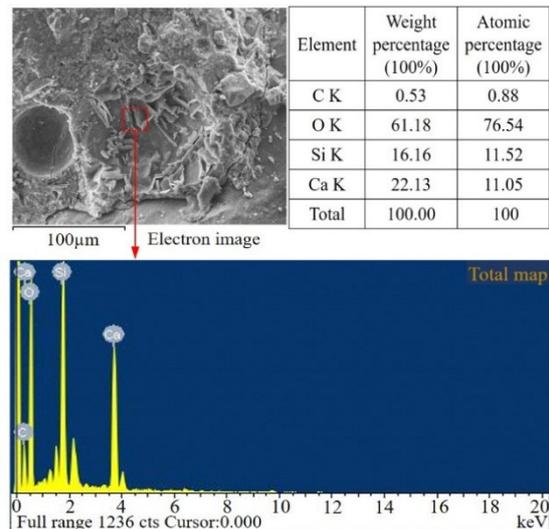
(a) C-CR0%-RA0%-Uncarbonated



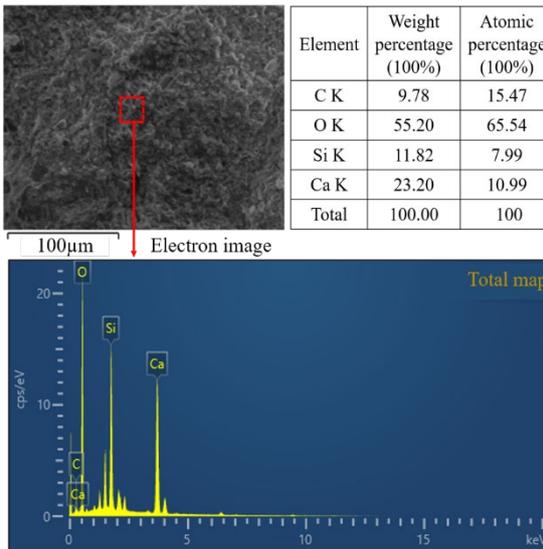
(b) C-CR0%-RA0%-7.5MPa



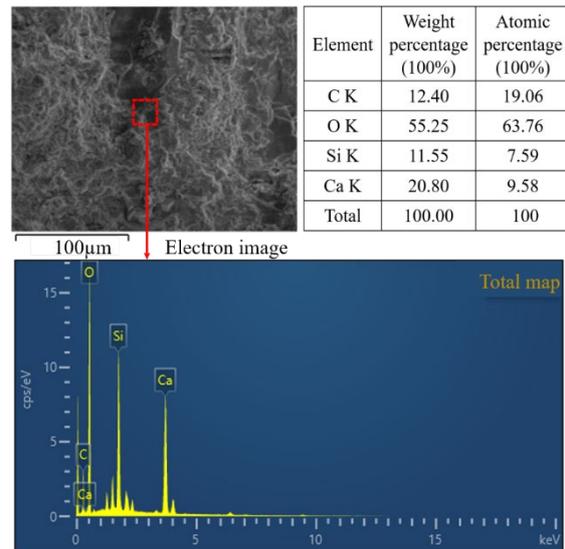
(c) C-CR30%-RA0%-Uncarbonated



(d) C-CR30%-RA0%-7.5MPa



(e) C-CR10%-RA100%-Uncarbonated



(f) C-CR10%-RA100%-7.5MPa

Fig. 16. EDX element analysis of LCRC specimens.

494

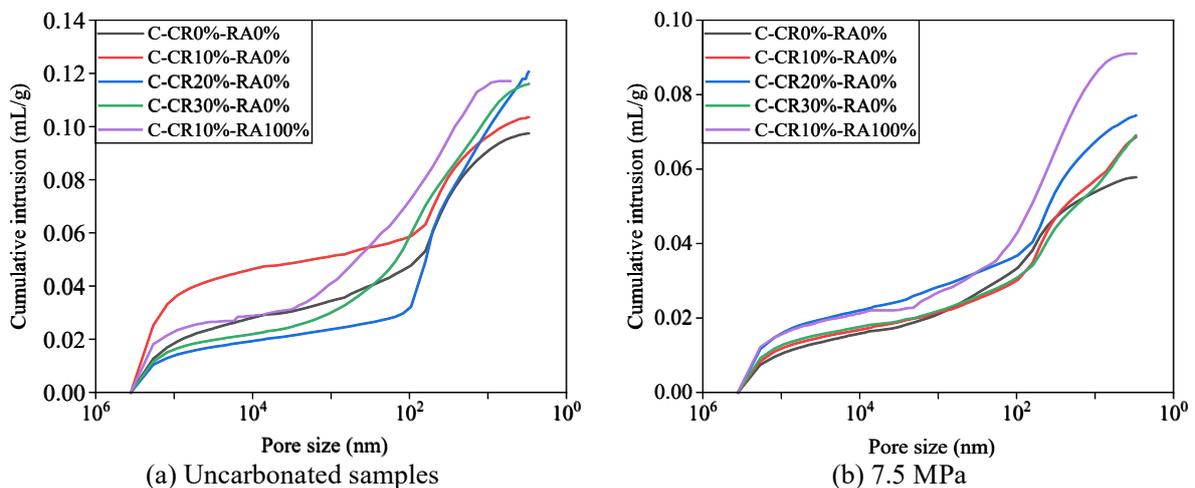
495 Under the uncarbonated condition, the Si/Ca atomic ratio in the 30% cement replacement sample
 496 increased by 46% compared to the 0% cement replacement LCRC sample. This primarily resulted from the
 497 reduced cement content that decreased $\text{Ca}(\text{OH})_2$ content in the system, while the pozzolanic reaction of the
 498 GGBS generated more C-S-H gel, further consuming free Ca elements. After carbonation at 7.5 MPa, the C
 499 atom ratio in the 0% cement replacement sample increased significantly by 97% compared to at the
 500 uncarbonated condition, while the 30% cement replacement sample only increased by 52%. This indicates
 501 that a higher cement replacement ratio limits the total amount of $\text{Ca}(\text{OH})_2$ available for carbonation reactions,
 502 thereby inhibiting CaCO_3 formation [73]. Furthermore, the C/Ca atom ratio in both the 0% and 30% cement
 503 replacement samples was higher than that in the uncarbonated samples after carbonation, indicating that C-
 504 S-H gel also participated in the carbonation reaction process alongside $\text{Ca}(\text{OH})_2$. EDX analysis of LCRC
 505 specimens with 100% RA replacement ratio and a 10% cement replacement ratio before and after carbonation
 506 shows that carbonation at 7.5 MPa increases the atomic percentage of C by up to 23.2%, confirming a

507 synergistic effect between the highly porous RA and elevated CO₂ pressure. Meanwhile, the Si/Ca atomic
 508 ratio remains relatively stable after carbonation, indicating that a low cement replacement ratio plays a
 509 regulatory role in maintaining alkali reserves and preventing structural imbalance caused by excessive
 510 consumption of Ca-bearing phases.

511 3.3.2. Porosity variation

512 To investigate the effects of cement and RA replacement ratios on the porosity of LCRC, representative
 513 specimens were selected, including those with a RA replacement ratio of 0% and cement replacement ratios
 514 of 0%, 10%, 20%, and 30%, as well as a specimen with a RA replacement ratio of 100% and a cement
 515 replacement ratio of 10%. The cumulative mercury intrusion curves of the LCRC specimens under the
 516 uncarbonated condition and external CO₂ pressures of 7.5 MPa are shown in Fig. 17, respectively, and the
 517 porosity test data for the LCRC before and after carbonation are summarized in Table 7.

518 Under the uncarbonated condition, the total porosity of the LCRC specimens containing 0% RA exhibits
 519 an increasing trend with higher cement replacement ratios. This is primarily attributed to the slower early-
 520 stage hydration rate of GGBS, which facilitates the formation of interconnected pores ranging from 1 to 5
 521 μm in size between unreacted particles, thereby enhancing the overall porosity. Analyzing different pore size
 522 distributions reveals that at 0% cement replacement, macropores (>1000 nm) accounted for 35% of the totals.
 523 When cement replacement reached 10%, 20%, and 30%, macropores percentage are 49%, 19%, and 26%,
 524 respectively. This is due to the physical filling effect of the GGBS, which refines the pore structure within
 525 the LCRC [69]. When the RA replacement ratio reaches 100%, the high porosity and water absorption
 526 characteristics of RA result in an initial porosity of the LCRC specimens that is 12.2% higher compared to
 527 specimens with a 0% RA replacement ratio. Under external CO₂ pressure of 7.5 MPa, the total porosity of
 528 the LCRC specimens with 0% RA replacement decreased by 28.94%-33.83% compared to at the
 529 uncarbonated state, with the most significant reduction observed in specimens with 30% cement replacement.
 530 For LCRC specimens with 100% RA replacement, the porosity decreased by 29.59% after carbonation,
 531 exceeding the reduction observed in specimens with 0% RA replacement. This is closely related to its highest
 532 initial porosity: more initial voids provided ample space for the precipitation and filling of the carbonation
 533 product CaCO₃, thereby amplifying the pore optimization effect of the carbonation reaction [42].



534 **Fig. 17.** MIP test results of LCRC before and after carbonation.

535 **Table 7** Porosity of LCRC specimens before and after carbonation.

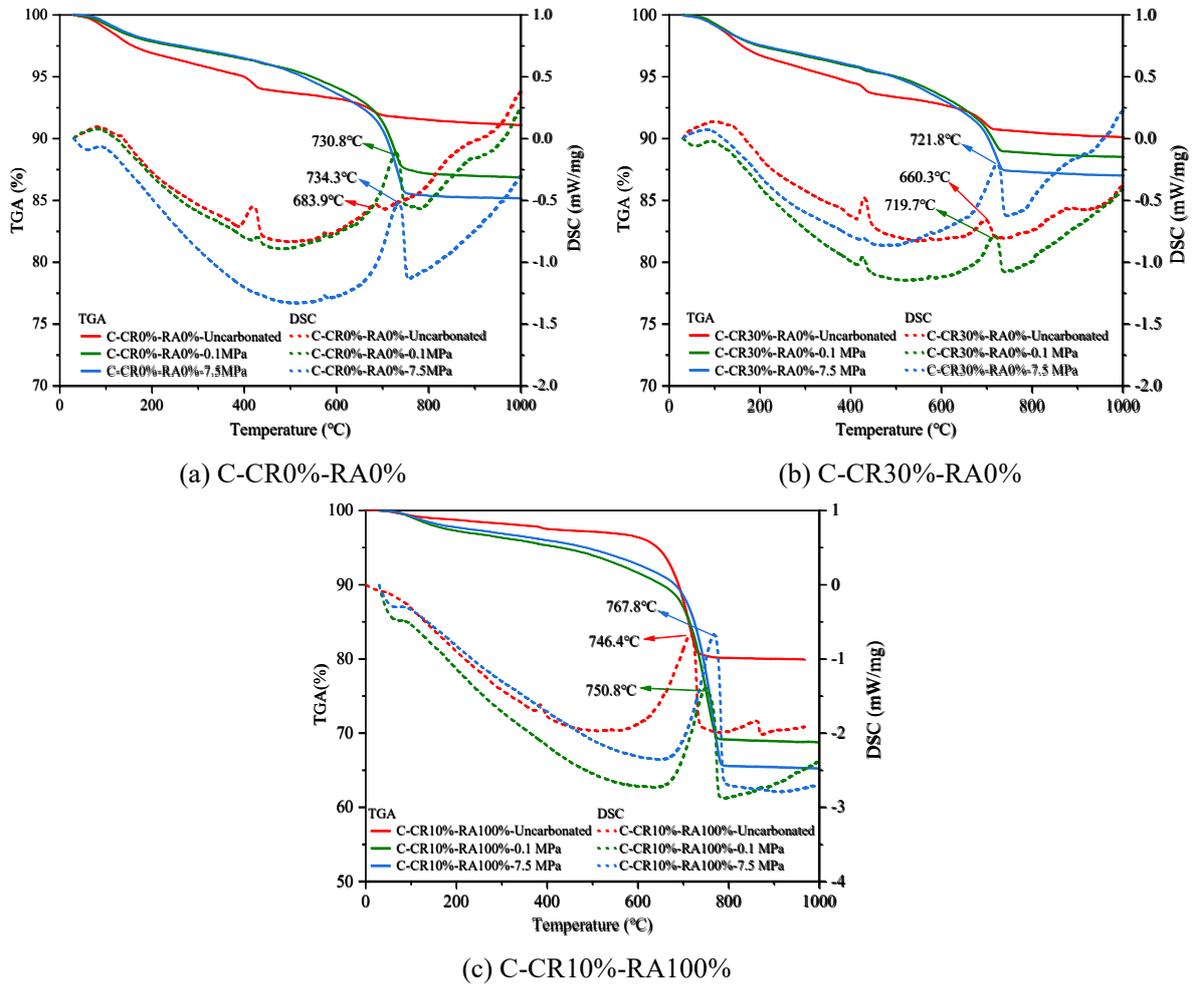
Groups	Porosity before carbonation	Porosity after carbonation	Ratio of decrement
C-CR0%-RA0%	18.41%	12.52%	32%
C-CR10%-RA0%	19.24%	13.66%	29%
C-CR20%-RA0%	20.08%	14.36%	28.49%
C-CR30%-RA0%	21.58%	14.28%	33.83%
C-CR10%-RA100%	23.89%	16.82%	29.59%

536 Remarks: Porosity values are the total intrusion porosity obtained from MIP measurements.

537 *3.3.3. Contents of Ca(OH)₂ and C-S-H*

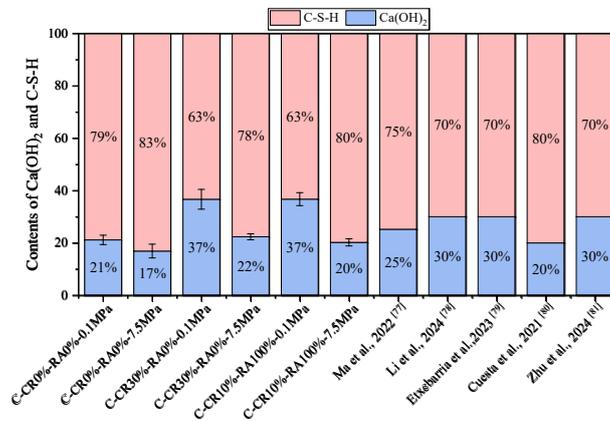
538 To investigate changes in CaCO₃ and Ca(OH)₂ content in the carbonate and uncarbonated zones of
 539 LCRC under varying CO₂ pressures, representative LCRC specimens were selected for testing: specimens
 540 with 0% RA replacement ratio and 0% or 30% cement replacement ratio, as well as specimens with 100%
 541 RA replacement ratio and 10% cement replacement ratio. Fig. 18 displays the TGA-DSC curves of these
 542 specimens after carbonation under external CO₂ pressures of 0.1 MPa and 7.5 MPa, as well as under the
 543 uncarbonated condition. The figure reveals that for all the carbonated specimens, the DSC curves exhibit
 544 significantly weakened peaks characterizing Ca(OH)₂ decomposition within the 400-500°C range, while the
 545 peaks indicating CaCO₃ decomposition within the 600-800°C range are markedly enhanced. This indicates
 546 that the carbonation reaction converts hydration products such as Ca(OH)₂ into CaCO₃ [74].

547 When the RA and cement replacement ratio were both 0% (Fig. 18a), the CaCO₃ mass fraction after 7.5
 548 MPa carbonation reached 8.26%, an increase of 18.3% compared to the 0.1 MPa condition, indicating that
 549 high pressure significantly promoted the formation of carbonation products. However, when the cement
 550 replacement ratio increased to 30% (Fig. 18b), the Ca(OH)₂ content in the uncarbonated sample was 1.03%,
 551 lower than the 1.28% observed in the sample with 0% cement replacement. This is primarily due to the
 552 reduced cement content and its slowed hydration rate, leading to decreased Ca(OH)₂ formation [75]. After
 553 carbonation at 7.5 MPa, the CaCO₃ content in this group of samples was 5.91%, representing a 28.5%
 554 decrease compared to samples with 0% cement replacement. This reduction is attributed to insufficient initial
 555 Ca(OH)₂ and the further consumption of Ca²⁺ by the GGBS pozzolanic reaction. At 100% RA replacement,
 556 the CaCO₃ mass fraction after 7.5 MPa carbonation reached 10.59%, a 28.2% increase compared to the 0%
 557 RA and cement replacement samples. This directly demonstrates that RA's high porosity provides efficient
 558 transport pathways for supercritical CO₂, whose synergistic interaction with the high-pressure environment
 559 significantly accelerates the deposition of carbonation products and pore filling, thereby establishing a
 560 microscopic foundation for enhancing macroscopic compressive strength.



561 **Fig. 18.** TGA-DSC results of LCRC before and after carbonation.

562 The Ca(OH)_2 and C-S-H contents in the cementitious material prior to carbonation were determined
 563 using the method. These values were compared with the C-S-H and Ca(OH)_2 contents calculated from other
 564 literature sources [76-81], as shown in Fig. 19. The results indicate that in most studies, the C-S-H content
 565 ranged from 60% to 83%, while Ca(OH)_2 content was found to be in the range of 17% to 40%.



566 **Fig. 19.** C-S-H and Ca(OH)_2 content in cementitious materials before carbonation.

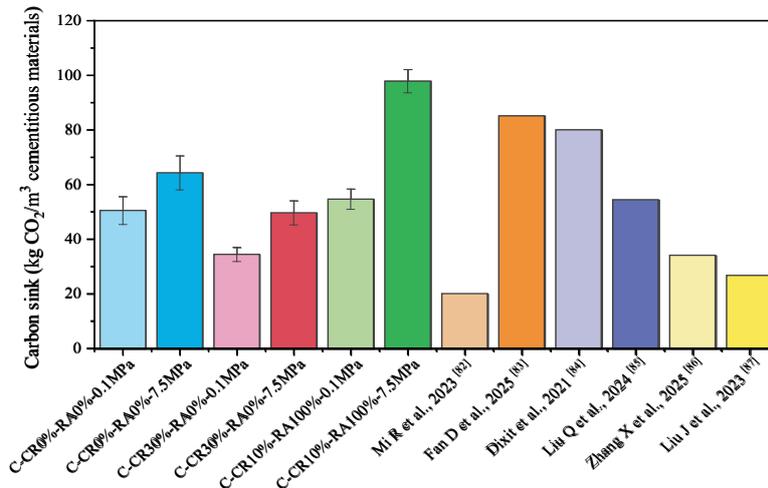
567

568 3.3.4. CO₂ uptake capacity

569 To evaluate the CO₂ uptake capacity of fully carbonated cementitious materials, this study calculated
 570 the CO₂ uptake capacity of LCRC using Eq. (10) based on the TGA mass loss in the temperature range of
 571 600-800°C shown in Fig. 18. The results indicate that under external CO₂ pressures of 0.1 MPa and 7.5 MPa,
 572 the average CO₂ uptake capacity of the LCRC specimens with cement replacement ratios of 0% and 30%
 573 was 42.23 kg/m³. Notably, when the RA replacement ratio reached 100% and the cement replacement ratio
 574 was 10%, the LCRC achieved its peak CO₂ uptake capacity of 96.09 kg/m³ under external CO₂ pressure of
 575 7.5 MPa. This represents a significant increase of approximately 49.6% compared to specimens with 0% RA
 576 and 0% cement replacement ratios. This indicates that the porous structure of RA provides an efficient
 577 transport pathway for supercritical CO₂, enabling its rapid penetration into the material interior. This
 578 simultaneously maximizes both CO₂ sequestration efficiency and microstructural reinforcement. At the same
 579 time, the material composition also plays a crucial regulatory role. Although increasing cement replacement
 580 ratios reduces carbon emissions during concrete production, it also diminishes its CO₂ uptake capacity. The
 581 CO₂ uptake capacity of LCRC with a 30% cement replacement ratio decreased by 22.8% after carbonation
 582 at 7.5 MPa compared to that of cement with a 0% replacement ratio. This indicates that a balance must be
 583 struck between maximizing carbon sequestration and reducing cement usage. Finally, the calculated results
 584 are compared in Fig. 20 with data from other studies [82-87].

$$m_{CO_2} = \frac{(D_{800} - D_{600}) \times m_c}{M_{CC}} \times M_{CO_2} \quad (10)$$

585 where m_{CO_2} is the mass of carbon dioxide absorbed per cubic meter of concrete, m_c is the mass per unit
 586 volume of LCRC, which is calculated as the bulk density (2100-2400 kg/m³) of the specific mix design, and
 587 M_{CO_2} is the molar mass of CO₂ (44 g/mol).



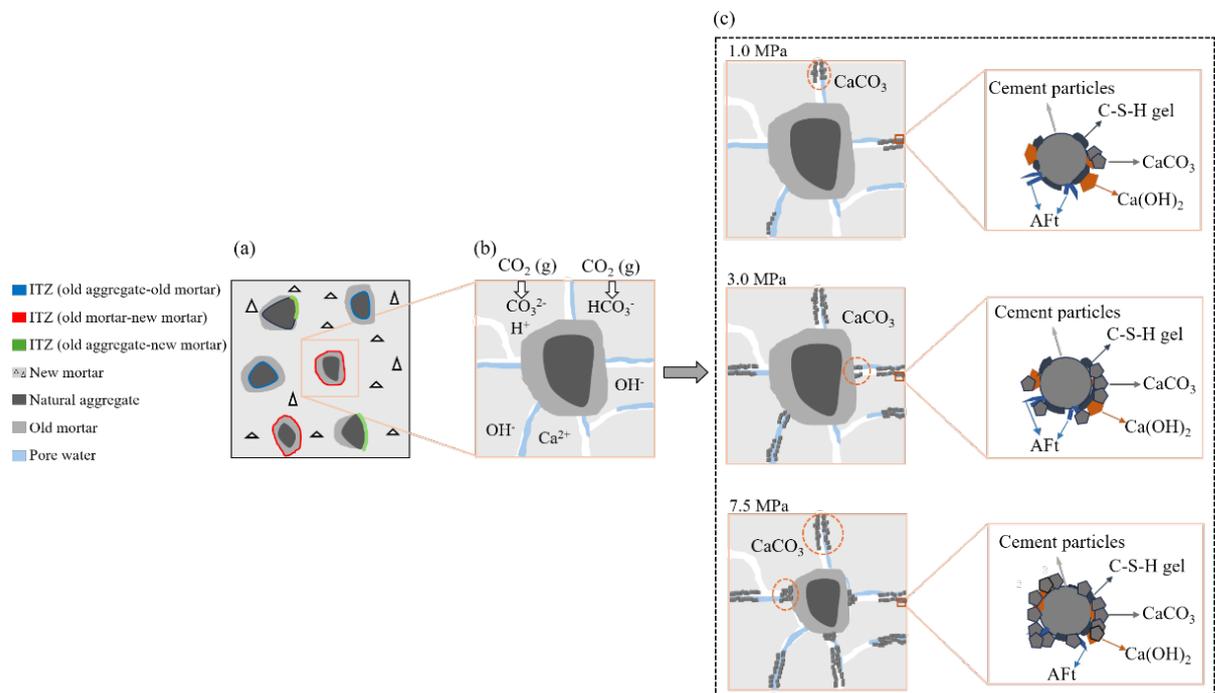
588
 589 **Fig. 20.** CO₂ uptake of cementitious materials after completely carbonation.

590 3.4. Accelerated carbonation mechanism of LCRC

591 In this study, the accelerated carbonation mechanism of LCRC under different CO₂ pressures was
 592 studied by means of SEM, MIP, and TGA-DSC, the essence of which is a pressure-driven physical and
 593 chemical coupling process [72] as shown in Fig. 21.

594 Increased external CO₂ pressure first creates a pressure gradient within the concrete, accelerating CO₂
 595 transport and elevating its concentration within pores. Simultaneously, this process promotes the migration
 596 of water within the concrete toward micro-pores, thereby exposing a larger gas-liquid reaction interface. At
 597 this expanded interface, the dissolution reaction rate of CO₂ significantly increases; where it dissolves to
 598 forms H₂CO₃, which then decompose into HCO₃⁻ and CO₃²⁻. The increase of CO₃²⁻ concentration not only
 599 accelerates the reaction with Ca²⁺ dissolved from Ca(OH)₂ in the pore solution but also promotes the
 600 decalcification and decomposition of C-S-H gel to produce more CaCO₃ and amorphous SiO₂. This was
 601 confirmed by the consumption of Ca(OH)₂, the formation of CaCO₃ and the increase of C/Ca atomic ratio in
 602 TGA-DSC and EDX analysis. The CaCO₃ crystals generated by the reaction effectively filled the pores and
 603 ITZ of the concrete. The MIP test showed that the total porosity could be reduced by about 30% after
 604 carbonation. The densification of this microstructure is the fundamental reason for the improvement of
 605 macroscopic compressive strength. However, high pressure and high carbonation may also lead to partial
 606 deconstruction of the C-S-H gel structure, and their effect on long-term durability needs further study.

607 Under atmospheric pressure conditions, concrete carbonation is primarily governed by CO₂ diffusion
 608 driven by concentration gradients, resulting in a relatively slow process dominated by surface reactions [24, 88,
 609 89]. In contrast, increasing external CO₂ pressure fundamentally alters the carbonation mechanism. Elevated
 610 CO₂ pressure significantly accelerates carbonation and increases its depth by enhancing the coupled
 611 processes of CO₂ transport, dissolution, and reaction. In this case, the pressure gradient, rather than the
 612 concentration gradient, becomes the dominant driving force, promoting CO₂ penetration and dissolution
 613 within the concrete matrix. This process facilitates the formation of CaCO₃ and pore-filling effects, thereby
 614 optimizing the microstructure and ultimately enhancing the mechanical properties of LCRC while enabling
 615 effective CO₂ sequestration.



616

617 **Fig. 21.** The carbonation reaction mechanism of LCRC under different external CO₂ pressure. (a) The phase
 618 composition of cementitious materials; (b) CO₂ transport and reactions in pore fluids; (c) The conversion

619 process of hydration products to CaCO₃.

620 **4. Conclusions**

621 This study investigated the effects of RA replacement ratio, cement replacement ratio, and external CO₂
622 pressure on the carbonation depth and compressive strength of LCRC. SEM, MIP, and TGA-DSC analyses
623 were conducted to characterize its microstructural evolution and reveal the accelerated carbonation
624 mechanism.

625 (1) Increasing RA replacement, cement replacement, and CO₂ pressure all deepen the carbonation depth,
626 of with CO₂ pressure was the dominant factor. At 1.0, 3.0, 5.0, and 7.5 MPa, the carbonation depth of the
627 LCRC increased by 19%, 50%, 83%, and 164%, respectively, compared with at 0.1 MPa.

628 (2) Increasing RA and cement replacement reduced compressive strength, while elevated CO₂ pressure
629 significantly enhanced the strength. Under the same pressure condition, the compressive strength increased
630 by 15%, 19%, 22%, 29%, and 47% respectively compared with the uncarbonated specimen.

631 (3) During carbonation, flocculent C-S-H gels in the matrix and interfacial zones was transformed into
632 CaCO₃, refining the pore structure and reducing total porosity by 28%-34%.

633 (4) TGA-DSC results indicated that pre-carbonation C-S-H and Ca(OH)₂ contents were 63%-83% and
634 17%-37%, respectively. The maximum CO₂ uptake capacity was about 96.09 kg CO₂/m³.

635 (5) Increasing CO₂ pressure accelerated gas transport and dissolution, promoted C-S-H decalcification
636 and CaCO₃ formation, and enhanced both pore densification and CO₂ uptake efficiency.

637 **CRedit authorship contribution statement**

638 **Hao Bao:** Conceptualization, Methodology, Supervision, Validation, Visualization, Writing-review &
639 editing. **Jie Wang:** Data curation, Investigation, Validation, Writing-original draft. **Gang Xu:** Supervision.
640 **Ruyu Wang:** Data curation, Investigation. **Mohamed Saafi:** Supervision. **Jianqiao Ye:** Supervision,
641 Visualization, Writing-review & editing.

642 **Declaration of Interest Statement**

643 The authors declare that they have no known competing financial interests or personal relationships that
644 could have appeared to influence the work reported in this article.

645 **Acknowledgements**

646 The authors are grateful for the financial support from the National Natural Science Foundation of China
647 (Grant No. 52308266), as well as the Future City Research Institute (FCRI) Flagship Program and the Royal
648 Society (Grant: IEC\NSFC\191350).

649 **Data availability statement**

650 Data can be available upon reasonable request to the corresponding author.

651 **References**

- 652 [1] R. Warburton, Global warming has concrete problem when it comes to CO₂, EcoRi News 25 (2019).
653 [2] N. Deutscher, Global cement production from 1990 to 2030 (in million metric tons), Statista GmbH
654 (Hamburg) (2019).
655 [3] Y. Ju, H. Zhang, D. Wang, et al., Effect of mineral admixtures on the resistance to sulfate attack of reactive

- 656 powder concrete, *Journal of Cleaner Production* 440 (2024) 140769.
657 <http://10.1016/j.jclepro.2024.140769>
- 658 [4] P.O. Awoyera, B.F. Britto, Foamed concrete incorporating mineral admixtures and pulverized ceramics:
659 Effect of phase change and mineralogy on strength characteristics, *Construction and Building Materials*
660 234 (2020) 117434. <http://10.1016/j.conbuildmat.2019.117434>
- 661 [5] L. Wu, Z. Sun, Y. Cao, Modification of recycled aggregate and conservation and application of recycled
662 aggregate concrete: A review, *Construction and Building Materials* 431 (2024).
663 <http://10.1016/j.conbuildmat.2024.136567>
- 664 [6] B.I. Oluleye, D.W.M. Chan, T.O. Olawumi, Barriers to circular economy adoption and concomitant
665 implementation strategies in building construction and demolition waste management: A PRISMA and
666 interpretive structural modeling approach, *Habitat International* 126 (2022).
667 <http://10.1016/j.habitatint.2022.102615>
- 668 [7] R.V. Silva, J. de Brito, R.K. Dhir, Use of recycled aggregates arising from construction and demolition
669 waste in new construction applications, *Journal of Cleaner Production* 236 (2019).
670 <http://10.1016/j.jclepro.2019.117629>
- 671 [8] A. Singh, X. Miao, X. Zhou, et al., Use of recycled fine aggregates and recycled powders in sustainable
672 recycled concrete, *Journal of Building Engineering* 77 (2023). <http://10.1016/j.job.2023.107370>
- 673 [9] S. Jamil, J. Shi, M. Idrees, Effect of various parameters on carbonation treatment of recycled concrete
674 aggregate using the design of experiment method, *Construction and Building Materials* 382 (2023)
675 131339. <http://10.1016/j.conbuildmat.2023.131339>
- 676 [10] F. Winnefeld, A. Leemann, A. German, et al., CO₂ storage in cement and concrete by mineral carbonation,
677 *Current Opinion in Green and Sustainable Chemistry* 38 (2022) 100672.
678 <http://10.1016/j.cogsc.2022.100672>
- 679 [11] Z. Duan, A. Singh, J. Xiao, et al., Combined use of recycled powder and recycled coarse aggregate
680 derived from construction and demolition waste in self-compacting concrete, *Construction and Building*
681 *Materials* 254 (2020) 119323. <http://10.1016/j.conbuildmat.2020.119323>
- 682 [12] G. Bai, C. Zhu, C. Liu, et al., An evaluation of the recycled aggregate characteristics and the recycled
683 aggregate concrete mechanical properties, *Construction and building materials* 240 (2020) 117978.
684 <http://10.1016/j.conbuildmat.2019.117978>
- 685 [13] B. Wang, L. Yan, Q. Fu, et al., A Comprehensive Review on Recycled Aggregate and Recycled
686 Aggregate Concrete, *Resources, Conservation and Recycling* 171 (2021) 105565.
687 <http://10.1016/j.resconrec.2021.105565>
- 688 [14] J. Wang, J. Zhang, D. Cao, Pore characteristics of recycled aggregate concrete and its relationship with
689 durability under complex environmental factors, *Construction and Building Materials* 272 (2021)
690 121642. <http://10.1016/j.conbuildmat.2020.121642>
- 691 [15] X. Liu, X. Xie, R. Liu, et al., Research on the durability of nano-SiO₂ and sodium silicate co-modified
692 recycled coarse aggregate (RCA) concrete, *Construction and Building Materials* 378 (2023) 131185.
693 <http://10.1016/j.conbuildmat.2023.131185>
- 694 [16] A.A. Bahraq, J. Jose, M. Shameem, et al., A review on treatment techniques to improve the durability of
695 recycled aggregate concrete: Enhancement mechanisms, performance and cost analysis, *Journal of*
696 *Building Engineering* 55 (2022) 104713. <http://10.1016/j.job.2022.104713>
- 697 [17] J. Xiao, B. Lei, C. Zhang, On carbonation behavior of recycled aggregate concrete, *Science China*
698 *Technological Sciences* 55(9) (2012) 2609-2616. <http://10.1007/s11431-012-4798-5>
- 699 [18] Y. Li, T. Fu, R. Wang, et al., An assessment of microcracks in the interfacial transition zone of recycled
700 concrete aggregates cured by CO₂, *Construction and Building Materials* 236 (2020) 117543.
701 <http://10.1016/j.conbuildmat.2019.117543>
- 702 [19] B. Lu, C. Shi, Z. Cao, et al., Effect of carbonated coarse recycled concrete aggregate on the properties
703 and microstructure of recycled concrete, *Journal of Cleaner Production* 233 (2019) 421-428.
704 <http://10.1016/j.jclepro.2019.05.350>
- 705 [20] B. Tang, M. Fan, Z. Yang, et al., A comparison study of aggregate carbonation and concrete carbonation
706 for the enhancement of recycled aggregate pervious concrete, *Construction and Building Materials* 371
707 (2023) 130797. <http://10.1016/j.conbuildmat.2023.130797>
- 708 [21] R. Silva, R. Neves, J. De Brito, et al., Carbonation behaviour of recycled aggregate concrete, *Cement*
709 *and Concrete Composites* 62 (2015) 22-32. <http://doi.org/10.1016/j.cemconcomp.2015.04.017>
- 710 [22] N.K. Bui, T. Satomi, H. Takahashi, Mechanical properties of concrete containing 100% treated coarse
711 recycled concrete aggregate, *Construction and Building Materials* 163 (2018) 496-507.
712 <http://10.1016/j.conbuildmat.2017.12.131>

- 713 [23] C. Liang, B. Pan, Z. Ma, et al., Utilization of CO₂ curing to enhance the properties of recycled aggregate
714 and prepared concrete: A review, *Cement and concrete composites* 105 (2020) 103446.
715 <http://10.1016/j.cemconcomp.2019.103446>
- 716 [24] W. Zuo, X. Zhao, J. Luo, et al., Investigating the impact of carbonation curing concentrations on the
717 mechanical properties and microstructure of recycled concrete, *Journal of Building Engineering* 96
718 (2024) 110465. <http://10.1016/j.job.2024.110465>
- 719 [25] S. Hang, L. Liang, Y. Huang, et al., Experimental evaluation of mechanical properties and microstructure
720 for recycled aggregate concrete collaboratively modified with nano-silica and mixed fibers,
721 *Construction and Building Materials* 403 (2023) 133125. <http://10.1016/j.conbuildmat.2023.133125>
- 722 [26] J. Yin, A. Kang, P. Xiao, et al., Influences of spraying sodium silicate based solution/slurry on recycled
723 coarse aggregate, *Construction and Building Materials* 377 (2023) 130924.
724 <http://10.1016/j.conbuildmat.2023.130924>
- 725 [27] S. Gao, Y. Ji, Z. Qin, et al., A comprehensive analysis of pore structures and performances of mineral
726 admixtures modified recycled aggregate concrete based on experiment and theory, *Construction and*
727 *Building Materials* 358 (2022) 129451. <http://10.1016/j.conbuildmat.2022.129451>
- 728 [28] R. Kurda, J. de Brito, J.D. Silvestre, Water absorption and electrical resistivity of concrete with recycled
729 concrete aggregates and fly ash, *Cement and Concrete Composites* 95 (2019) 169-182.
730 <http://10.1016/j.cemconcomp.2018.10.004>
- 731 [29] N. Bahrami, M. Zohrabi, S.A. Mahmoudy, et al., Optimum recycled concrete aggregate and micro-silica
732 content in self-compacting concrete: Rheological, mechanical and microstructural properties, *Journal of*
733 *building Engineering* 31 (2020) 101361. <http://10.1016/j.job.2020.101361>
- 734 [30] Y. Wu, H. Mehdizadeh, K.H. Mo, et al., High-temperature CO₂ for accelerating the carbonation of
735 recycled concrete fines, *Journal of Building Engineering* 52 (2022) 104526.
736 <http://10.1016/j.job.2022.104526>
- 737 [31] C. Wang, J. Wang, X. Liu, et al., Orthogonal experimental design for compressive strength of recycled
738 coarse aggregate concrete with silica fume-slag-fly ash hybrid micro-powders, *Construction and*
739 *Building Materials* 408 (2023) 133669. <http://10.1016/j.conbuildmat.2023.133669>
- 740 [32] M.S. Rais, R.A. Khan, Experimental investigation on the strength and durability properties of bacterial
741 self-healing recycled aggregate concrete with mineral admixtures, *Construction and Building Materials*
742 306 (2021) 124901. <http://10.1016/j.conbuildmat.2021.124901>
- 743 [33] J. Sun, P. Zhang, Effects of different composite mineral admixtures on the early hydration and long-term
744 properties of cement-based materials: A comparative study, *Construction and Building Materials* 294
745 (2021) 123547. <http://10.1016/j.conbuildmat.2021.123547>
- 746 [34] J. Wang, Z. Che, K. Zhang, et al., Performance of recycled aggregate concrete with supplementary
747 cementitious materials (fly ash, GBFS, silica fume, and metakaolin): Mechanical properties, pore
748 structure, and water absorption, *Construction and Building Materials* 368 (2023) 130455.
749 <http://10.1016/j.conbuildmat.2023.130455>
- 750 [35] L.A. Qureshi, B. Ali, A. Ali, Combined effects of supplementary cementitious materials (silica fume,
751 GGBS, fly ash and rice husk ash) and steel fiber on the hardened properties of recycled aggregate
752 concrete, *Construction and Building Materials* 263 (2020) 120636.
753 <http://10.1016/j.conbuildmat.2020.120636>
- 754 [36] L. Qin, X. Gao, T. Chen, Influence of mineral admixtures on carbonation curing of cement paste,
755 *Construction and Building Materials* 212 (2019) 653-662. <http://10.1016/j.conbuildmat.2019.04.033>
- 756 [37] T. Chen, M. Bai, X. Gao, Carbonation curing of cement mortars incorporating carbonated fly ash for
757 performance improvement and CO₂ sequestration, *J. CO₂ Util.* 51 (2021) 101633.
758 <http://10.1016/j.jcou.2021.101633>
- 759 [38] D. Wang, T. Noguchi, T. Nozaki, Increasing efficiency of carbon dioxide sequestration through high
760 temperature carbonation of cement-based materials, *Journal of Cleaner Production* 238 (2019) 117980.
761 <http://10.1016/j.jclepro.2019.117980>
- 762 [39] P. Liu, Z. Yu, Y. Chen, Carbonation depth model and carbonated acceleration rate of concrete under
763 different environment, *Cement and Concrete Composites* 114 (2020) 103736.
764 <http://10.1016/j.cemconcomp.2020.103736>
- 765 [40] T. Chen, X. Gao, Effect of carbonation curing regime on strength and microstructure of Portland cement
766 paste, *J. CO₂ Util.* 34 (2019) 74-86. <http://doi.org/10.1016/j.jcou.2019.05.034>
- 767 [41] X. Xian, D. Zhang, H. Lin, et al., Ambient pressure carbonation curing of reinforced concrete for CO₂
768 utilization and corrosion resistance, *J. CO₂ Util.* 56 (2022) 101861. <http://10.1016/j.jcou.2021.101861>
- 769 [42] H. Bao, T. Wang, M. Yu, et al., Experimental study and numerical modeling of supercritical carbonation

- 770 of steel fiber reinforced concrete, *Construction and Building Materials* 476 (2025) 141249.
771 <http://10.1016/j.conbuildmat.2025.141249>
- 772 [43] Q. Liu, A. Cheng, A. Singh, et al., Performance enhancement of recycled concrete through carbonation
773 during ready-mix and curing, *Construction and Building Materials* 458 (2025) 139665.
774 <http://10.1016/j.conbuildmat.2024.139665>
- 775 [44] B.J. Zhan, D.X. Xuan, C.S. Poon, et al., Effect of curing parameters on CO₂ curing of concrete blocks
776 containing recycled aggregates, *Cement and Concrete Composites* 71 (2016) 122-130.
777 <http://10.1016/j.cemconcomp.2016.05.002>
- 778 [45] S. Ndiaye, J.-S. Condoret, F. Bourgeois, et al., High-pressure carbonation of mortar as a model for
779 recycled concrete aggregates, *The Journal of Supercritical Fluids* 198 (2023) 105932.
780 <http://10.1016/j.supflu.2023.105932>
- 781 [46] GB/T 14685-2022, Pebble and crushed stone for construction. 2022-04-15. (in chinese)
- 782 [47] D. Sun, W. Huang, K. Liu, et al., Effect of the moisture content of recycled aggregate on the mechanical
783 performance and durability of concrete, *Materials* 15(18) (2022) 6299. <http://10.3390/ma15186299>
- 784 [48] A. Salas_Montoya, C.-W. Chung, B.E. Mira_Rada, Interaction effect of recycled aggregate type,
785 moisture state, and mixing process on the properties of high-performance concretes, *Case Studies in*
786 *Construction Materials* 18 (2023) e02208. <http://10.1016/j.cscm.2023.e02208>
- 787 [49] Y. Wu, Z. Qi, M. Niu, et al., Effect of Moisture Condition of Brick–Concrete Recycled Coarse Aggregate
788 on the Properties of Concrete, *Materials* 15(20) (2022). <http://10.3390/ma15207204>
- 789 [50] GB/T 25177-2010, Recycled coarse aggregate for concrete. 2010-09-26. (in chinese)
- 790 [51] J. Liu, K. Ma, J. Shen, et al., Influence of recycled concrete aggregate enhancement methods on the
791 change of microstructure of ITZs in recycled aggregate concrete, *Construction and Building Materials*
792 371 (2023) 130772. <http://10.1016/j.conbuildmat.2023.130772>
- 793 [52] P. Jiang, F. Wang, N. Li, et al., Polyurethane foam lightweight concrete: Preparation, CO₂ fixation
794 properties and mechanism, *J. CO₂ Util.* 97 (2025) 103109. <http://10.1016/j.jcou.2025.103109>
- 795 [53] B. Graybeal, M. Davis, Cylinder or cube: strength testing of 80 to 200 MPa (11.6 to 29 ksi) ultra-high-
796 performance fiber-reinforced concrete, *ACI Materials Journal* 105(6) (2008) 603. <http://10.14359/20202>
- 797 [54] S.J.A. Hosseini, A.B.A. Rahman, Effects of spiral confinement to the bond behavior of deformed
798 reinforcement bars subjected to axial tension, *Engineering structures* 112 (2016) 1-13.
799 <http://10.1016/j.engstruct.2015.12.038>
- 800 [55] GB/T 50080-2016, Standard for test method of performance on ordinary fresh concrete 2016-08-18.
801 (in chinese)
- 802 [56] H. Gao, H. Liao, M. Wang, et al., Reinforcing the physicochemical properties of concrete through
803 synergism of CO₂ curing and Ca(OH)₂ solution drenching, *Construction and Building Materials* 280
804 (2021) 122546. <http://10.1016/j.conbuildmat.2021.122546>
- 805 [57] S. Tang, J. Huang, L. Duan, et al., A review on fractal footprint of cement-based materials, *Powder*
806 *Technology* 370 (2020) 237-250. <http://10.1016/j.powtec.2020.05.065>
- 807 [58] Y. Yahui, X. Gang, T. Bin, Carbonation characteristics of cement-based materials under the uniform
808 distribution of pore water, *Construction and Building Materials* 275 (2021) 121450.
809 <http://10.1016/j.conbuildmat.2020.121450>
- 810 [59] S. von Greve-Dierfeld, B. Lothenbach, A. Vollpracht, et al., Understanding the carbonation of concrete
811 with supplementary cementitious materials: a critical review by RILEM TC 281-CCC, *Materials and*
812 *structures* 53(6) (2020) 136. <http://10.1617/s11527-020-01558-w>
- 813 [60] H. Bao, R. Wang, Y. Yang, et al., Effect of pore water saturation on the supercritical carbonation of
814 cementitious materials, *Materials and Structures* 58(6) (2025) 1-22. [http://10.1617/s11527-025-02740-](http://10.1617/s11527-025-02740-8)
815 [8](http://10.1617/s11527-025-02740-8)
- 816 [61] GB/T 50082-2024, Standard for test methods of long-term performance and durability of concrete. 2024-
817 08-23. (in chinese)
- 818 [62] GB/T 50081-2019, Standard for test methods of concrete physical and mechanical properties. 2019-06-
819 19. (in chinese)
- 820 [63] J. Lapeyre, S.A. Ponduru, M. Okoronkwo, et al., Hydration of high-alumina calcium aluminate cements
821 with carbonate and sulfate additives, *Journal of Thermal Analysis and Calorimetry* 147(10) (2021) 5575-
822 5587. <http://10.1007/s10973-021-10939-4>
- 823 [64] N. Russo, F. Lollini, Effect of carbonated recycled coarse aggregates on the mechanical and durability
824 properties of concrete, *Journal of Building Engineering* 51 (2022). <http://10.1016/j.jobbe.2022.104290>
- 825 [65] N. Hosseinzadeh, P. Suraneni, Synergistic effects of air content and supplementary cementitious
826 materials in reducing damage caused by calcium oxychloride formation in concrete, *Cement and*

- 827 Concrete Composites 122 (2021) 104170. <http://10.1016/j.cemconcomp.2021.104170>
- 828 [66] C. Liang, J. Bao, F. Gu, et al., Determining the importance of recycled aggregate characteristics affecting
829 the elastic modulus of concrete by modeled recycled aggregate concrete: Experiment and numerical
830 simulation, Cement and Concrete Composites 162 (2025) 106118.
831 <http://10.1016/j.cemconcomp.2025.106118>
- 832 [67] X. Bao, L. Wang, Y. Ding, et al., Study on the mechanical properties and carbonation of concrete under
833 different phases of CO₂, Journal of Building Engineering 113 (2025) 114150.
834 <http://10.1016/j.jobe.2025.114150>
- 835 [68] B. Li, A. Gao, Y. Li, et al., Effect of silica fume content on the mechanical strengths, compressive stress–
836 strain behavior and microstructures of geopolymeric recycled aggregate concrete, Construction and
837 Building Materials 384 (2023) 131417. <http://10.1016/j.conbuildmat.2023.131417>
- 838 [69] M. Sumesh, U.J. Alengaram, M.Z. Jumaat, et al., Incorporation of nano-materials in cement composite
839 and geopolymer based paste and mortar – A review, Construction and Building Materials 148 (2017)
840 62-84. <http://10.1016/j.conbuildmat.2017.04.206>
- 841 [70] S. Tejas, D. Pasla, Assessment of mechanical and durability properties of composite cement-based
842 recycled aggregate concrete, Construction and Building Materials 387 (2023) 131620.
843 <http://10.1016/j.conbuildmat.2023.131620>
- 844 [71] S. Omary, E. Ghorbel, G. Wardeh, Relationships between recycled concrete aggregates characteristics
845 and recycled aggregates concretes properties, Construction and Building Materials 108 (2016) 163-174.
846 <http://10.1016/j.conbuildmat.2016.01.042>
- 847 [72] H. Bao, G. Xu, M. Yu, et al., Evolution of ITZ and its effect on the carbonation depth of concrete under
848 supercritical CO₂ condition, Cement and Concrete Composites 126 (2022) 104336.
849 <http://10.1016/j.cemconcomp.2021.104336>
- 850 [73] S.N. Abbas, M.I. Qureshi, An investigation of mechanical properties of Self Compacting Concrete (SCC)
851 after addition of Ground Granulated Blast Furnace Slag (GGBFS) and Fly Ash (FA), Materials
852 Chemistry and Physics: Sustainability and Energy 2 (2025) 100008.
853 <http://10.1016/j.macse.2025.100008>
- 854 [74] M. Liu, S. Hong, Y. Wang, et al., Compositions and microstructures of hardened cement paste with
855 carbonation curing and further water curing, Construction and Building Materials 267 (2021) 121724.
856 <http://10.1016/j.conbuildmat.2020.121724>
- 857 [75] P. He, S. Drissi, X. Hu, et al., Investigation on the influential mechanism of FA and GGBS on the
858 properties of CO₂-cured cement paste, Cement and Concrete Composites 142 (2023) 105186.
859 <http://10.1016/j.cemconcomp.2023.105186>
- 860 [76] K.Kobayashi, K.Suzuki, Carbonation of concrete structures and decomposition of C-S-H, Cement and
861 Concrete Resrarch 24 (1993) 55-61. [http://10.1016/0008-8846\(94\)90082-5](http://10.1016/0008-8846(94)90082-5)
- 862 [77] Y. Ma, W. Li, M. Jin, et al., Influences of leaching on the composition, structure and morphology of
863 calcium silicate hydrate (C–S–H) with different Ca/Si ratios, Journal of Building Engineering 58 (2022)
864 105017. <http://10.1016/j.jobe.2022.105017>
- 865 [78] X. Li, Y. Fan, C. Wu, et al., Advances in Molecular Dynamics-Based Characterization of Water and Ion
866 Adsorption and Transport in CSH Gels, Polymers 16(23) (2024) 3285. <http://10.3390/polym16233285>
- 867 [79] X.M. Aretxabaleta, J. López-Zorrilla, I. Etxebarria, et al., Multi-step nucleation pathway of CSH during
868 cement hydration from atomistic simulations, Nature Communications 14(1) (2023) 7979.
869 <http://10.1038/s41467-023-43500-y>
- 870 [80] A. Cuesta, I. Santacruz, G. Angeles, et al., Local structure and Ca/Si ratio in CSH gels from hydration
871 of blends of tricalcium silicate and silica fume, Cement and Concrete Research 143 (2021) 106405.
872 <http://10.1016/j.cemconres.2021.106405>
- 873 [81] Y. Zhu, Q. Zhu, F. Qi, Research Progress on the Preparation and Comprehensive Utilization of Calcium
874 Silicate Hydrate from Solid Waste, Bulletin of the Chinese Ceramic Society 43(2) (2024). (in chinese)
875 <http://10.16552/j.cnki.issn1001-1625.2024.02.022>
- 876 [82] R. Mi, T. Yu, C.S. Poon, Feasibility of utilising porous aggregates for carbon sequestration in concrete,
877 Environmental Research 228 (2023) 115924. <http://10.1016/j.envres.2023.115924>
- 878 [83] D. Fan, J.-X. Lu, X.-S. Lv, et al., Carbon capture and storage CO₂ foam concrete towards higher
879 performance: Design, preparation and characteristics, Cement and Concrete Composites 157 (2025)
880 105925. <http://10.1016/j.cemconcomp.2025.105925>
- 881 [84] A. Dixit, H. Du, S.D. Pang, Carbon capture in ultra-high performance concrete using pressurized CO₂
882 curing, Construction and Building Materials 288 (2021) 123076.
883 <http://10.1016/j.conbuildmat.2021.123076>

- 884 [85] Q. Liu, Y. Wang, C. Sun, et al., Carbon sequestration and mechanical properties of foam concrete based
885 on red mud pre-carbonation and CO₂ foam bubbles, *Construction and Building Materials* 426 (2024)
886 135961. <http://10.1016/j.conbuildmat.2024.135961>
- 887 [86] X. Zhang, S. Liu, G. Cai, et al., Scaled model test into the spatial variability and carbon storage potential
888 of carbon sequestration foamed concrete as a subgrade filler, *Construction and Building Materials* 471
889 (2025) 140718. <http://10.1016/j.conbuildmat.2025.140718>
- 890 [87] J. Liu, G. Liu, W. Zhang, et al., A new approach to CO₂ capture and sequestration: A novel carbon capture
891 artificial aggregates made from biochar and municipal waste incineration bottom ash, *Construction and*
892 *Building Materials* 398 (2023) 132472. <http://10.1016/j.conbuildmat.2023.132472>
- 893 [88] X. Chen, S. Li, M. Hu, et al., Early age accelerated carbonation of cementitious materials surface in low
894 CO₂ concentration condition, *Construction and Building Materials* 445 (2024).
895 <http://10.1016/j.conbuildmat.2024.137975>
- 896 [89] Y. Jiang, Z. Ma, Y. Gao, et al., A review on the impact of water in accelerated carbonation: Implications
897 for producing sustainable construction materials, *Cement and Concrete Composites* 157 (2025).
898 <http://10.1016/j.cemconcomp.2024.105902>