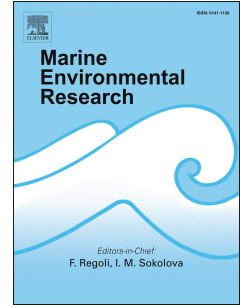


# Journal Pre-proof

Interactions between vegetation, sedimentation and flood inundation levels in wetlands

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1 **Interactions between vegetation, sedimentation and flood inundation**  
2 **levels in wetlands**

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19

20 **Keywords:** Peak flow hydrodynamic regimes, laboratory flume experiments, natural  
21 vegetation, sedimentation patterns

22 **Highlights:**

23 Aquatic emergent vegetation exerts drag on peak flows in inundated wetland systems

24 High-inundated wetlands produce flow resistance against peak flows, enhancing  
25 sedimentation close to the source

26 In low-inundated wetlands, inertia dominates transporting sediment far from the source

27

## 28 **Abstract**

29 Wetlands produce key ecosystem services to mitigate the impacts of peak flows caused  
30 by pluvial or fluvial floods or storm surges. Sediment floods were characterized by a peak  
31 flow flowing over a simulated wetland, populated with two natural species. Floods have  
32 been drawn as flows of height  $H$ , into waters of height  $h$ , where  $H > h$ . Peak flow along  
33 the flume passed through: peak flow adjustment; peak flow; drag-dominated peak flow;  
34 and gravity current regimes. For high inundation wetland levels, settling rates of coarse  
35 and fine sediment were similar during the peak flow regime. At larger distances,  
36 sedimentation decreased monotonically, with higher sedimentation of fine particles. For  
37 low inundation levels, the sedimentation rate during the drag-dominated peak flow  
38 regime was higher for coarse particles. Vegetation decreased the inundation level  
39 needed for enhancing sedimentation. Our study then adds practical knowledge at  
40 considering that the synergies between the vegetation and the inundation level may  
41 enhance wetland services such as the mitigation of pluvial, fluvial or coastal floodings.

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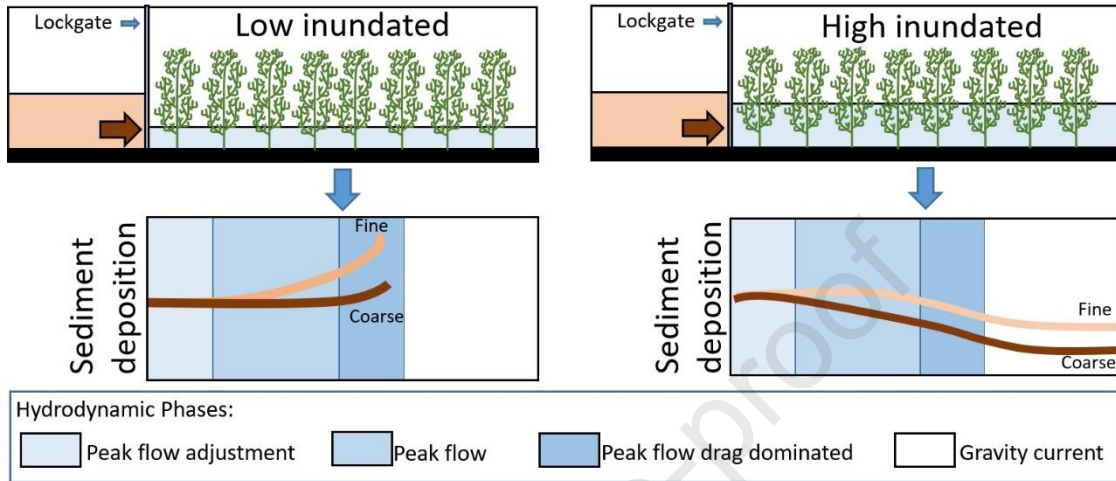
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45

46 Graphical Abstract

47

Peak flows in wetlands



48

## 49 **1 Introduction**

50 Wetlands are very productive environments with high levels of biodiversity providing  
51 habitat for a wide variety of species, many of which are exclusive to these environments  
52 (Balwan and Kour, 2021; Larson and Adamus, 1989). They also serve as coastal  
53 protection structures via hydrological and biogeochemical processes (Junk et al., 2013),  
54 contributing to their protection against floods, preventing soil erosion (Barcelona et al.,  
55 2018; Lo et al., 2017), promoting sedimentation and soil stabilization (Montakhab et al.,  
56 2012), and maintaining water quality through retention, removal and transformation of  
57 nutrients. In addition, wetlands contribute to the mitigation of climate change, since  
58 they are carbon sinks, holding between 20 and 30% of the Earth's total soil carbon  
59 (Nahlik and Fennessy, 2016) and reduce the consequences of the increased frequency  
60 and intensity of extreme weather phenomena (Fairchild et al., 2021; Jones et al., 2020;).

61 Wetlands are globally subject to anthropogenic impacts that harm their conservation,  
62 cause their regression and, in extreme cases, lead to their disappearance (Gardner and  
63 Finalyson, 2018). These impacts are caused by the drainage and conversion of the land  
64 for agricultural activities (Luo et al., 2022), the expansion of urbanization and tourist  
65 development (Chen et al., 2023), water pollution (Fu et al., 2023), and the introduction  
66 of invasive species (Choi et al. 2021; Shin et al., 2022). Climate change also represents a  
67 threat to the future of coastal wetlands (Duarte, 2002; Marbà and Duarte, 2010),  
68 especially due to water scarcity and society pressures (Lefebvre et al., 2019) with society  
69 development in coastline areas exacerbating water stress (Davidson, 2014).

70 The increase in global warming will impact coastal areas with an increase in sea level  
71 and erosive processes (Gedan et al., 2009; Reed et al., 2018), and an increase in the

72 frequency of hydrometeorological phenomena such as coastal flooding and maritime  
73 storms (Hoggart et al., 2014). Inland wetlands are also to be increasingly affected by  
74 pluvial and fluvial floods (Kundzewicz and Pinskiwar, 2020).

75 Floods in wetlands can be caused by the effect of extreme rainfalls in rivers or by the  
76 effect of storms in coastal areas. When there is a high rainfall intensity in a short period  
77 of time that exceeds the capacity of infiltration it can result in a pluvial inundation  
78 (Sauer, J., 2022) or if water level rise exceeding the riverbank it can result in a fluvial  
79 inundation. Sometimes, coastal wetlands are inundated by the abnormal rise in  
80 seawater level during a storm (*Storm surge*) (Wamsley et al., 2010). In other occasions,  
81 several hydrological processes combine leading to a complex pattern in the flooding  
82 level of the saltmarsh area, compromising also the future of the ecosystem (McGrath et  
83 al., 2023). All these inundation processes are of concern to riparian and coastal  
84 communities and can result in increased runoff rates, volumes and peak flows which can  
85 be reduced by wetland ecosystems helping therefore to decrease the impact of  
86 downstream flooding (Babbar-Sebens et al., 2013; Lemke and Richmond, 2009; Mistch  
87 and Day, 2006).

88 Vegetation in wetlands helps reduce the speed of floods as they flow over the landscape,  
89 and they can also provide immense water storage benefits while slowing water  
90 circulation to further reduce the height of floods and associated erosion rates (Healey  
91 et al., 2023; Sheng et al., 2022). When plant density, frontal area and stiffness increase,  
92 it results in a reduction of mean flow speed (Kadlec 1990; Västilä and Järvelä 2014),  
93 resulting in both the enhancement of sediment deposition (Soler et al., 2017) along with  
94 a reduction of sediment resuspension (Leonard and Luther 1995; Ward et al., 1984). In

95 degraded wetlands with less vegetation, there would be less protection against erosion  
96 therefore less sedimentation is expected to occur (Bouma et al., 2009), which would  
97 worsen the state of the soil in the wetland and would further hinder the growth of the  
98 vegetation itself. Wetlands provide immense water storage benefits while slowing  
99 water circulation to further reduce the height of floods and associated erosion rates  
100 (Healey et al., 2023; Sheng et al., 2022). Wetlands are also threatened by the effect of  
101 urban projects. Rojas et al. (2022) analyzed the flood mitigation ecosystem service of a  
102 coastal wetland in central Chile. They found that the occupation of wetland areas in  
103 central Chile is nearly a 50% projected to further rise, therefore decreasing any potential  
104 role in the flood mitigation. Flood hazard maps, for an extreme return period (500  
105 years), show that the water volume stored by a wetland would decrease by more than  
106 38% and the flooded area of the wetland by 30% (Rojas et al., 2022). It is then important  
107 to restore wetlands and flood plains (Brémond et al., 2013) as a policy during flood risk  
108 management (Ferreira et al., 2023). Wetlands restoration, through planting or seeding  
109 has had significantly improvement on attenuating floods peaks (Dakhlalla and Parajuli,  
110 2016; Faulkner et al., 2011). The protecting of existing wetlands has been found to  
111 provide the highest return on social investment (Pattison-Williams et al., 2018).

112 Considering that extreme climatic events such as drought, flooding and storms are  
113 expected to occur more frequently worldwide due to ongoing climate change, and given  
114 that wetlands play an important role in flood abatement (Acreman and Holden, 2013),  
115 soaking up and storing floodwater (Jessop et al., 2015), it is of great interest to study  
116 how wetlands can help to mitigate the impacts of peak flows caused by pluvial or fluvial  
117 floods or storm surges. This issue is addressed in the present study by performing  
118 laboratory flume peak flow experiments to simulate flood processes with the objectives

119 of 1) studying the effects of different levels of inundation in inundated wetlands on the  
120 hydrodynamics and the associated sediment transport of a peak flow; and 2) studying  
121 the effect of vegetation in modifying the development of a peak flow and the impact in  
122 the sedimentation along the flooding development. It is generally acknowledged that  
123 wetlands have the potential to reduce flooding effects, but the magnitude of  
124 attenuation is the subject of debate and difficult to assess. The present study is focused  
125 on study the effect of peak flows in flooded wetlands with an experimental inundated  
126 vegetated area of height  $h$  receiving a peak flow of height  $H$ . The experiments carried  
127 out in this study, where  $H > h$ , complementing those where  $H = h$  concerning gravity  
128 sediment-laden currents previously reported largely in the literature (Soler et al. 2020,  
129 2021), and those where  $h = 0$  corresponding to peak flows in dry wetlands (Hooke, 2019;  
130 Laronne and Reid, 1996).

131 This paper adds knowledge on the impacts of both the wetland inundation level and the  
132 vegetation water resistance on hydrodynamics and sedimentary patterns in front of a  
133 peak flow. This represents an advancement over previous studies of wetland benefits in  
134 front of flooding events and can provide management strategies of natural wetlands  
135 (Ferreira et la., 2023) or even a better optimisation of wetlands designed for natural  
136 based solutions in order to minimize the peak floods events.

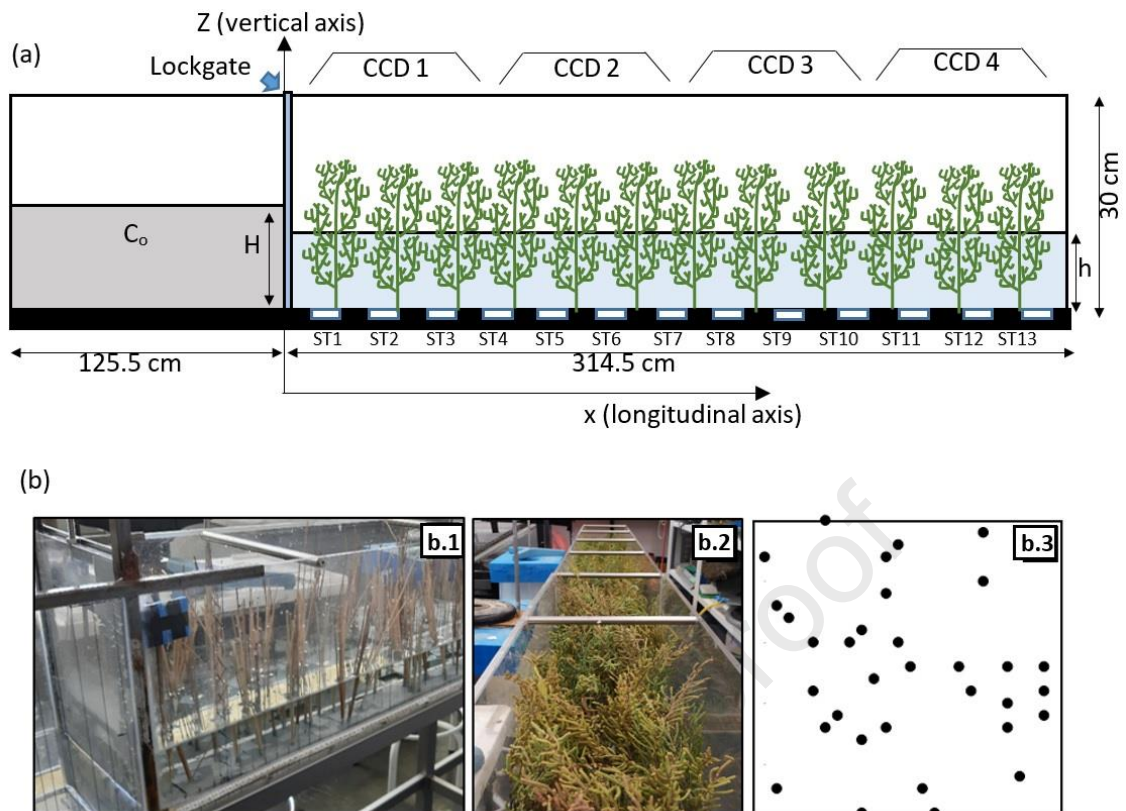
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## 138 **2 Methodology**

### 139 **2.1 Experimental setup**



140 Experiments were conducted in a 4.40 m long, 0.30 m high and 0.30 m wide  
141 methacrylate flume that was separated into two sections by a removable vertical lock  
142 gate. The shorter section (to the left in Fig. 1A), of 1.25 m length, acted as a reservoir  
143 containing sediment-laden water that would form the peak flow. The longer section (to  
144 the right in Fig. 1A) of 3.15 m length simulated the flooded environment where the  
145 interaction of the peak flow with the vegetation occurred. The initial water height,  $H$ , in  
146 the shorter section ranged from 25% to 50% of the canopy plants height, i.e. 6 to 12 cm  
147 and with an initial volume  $V_0$  (see Table 1). The initial height of the longer section where  
148 the peak flow developed,  $h$ , ranged from 12.5% to 37.5% of the canopy plants height,  
149 i.e. 3 and 9 cm (Table 1). These inundation levels are in the range of those observed in  
150 natural wetlands of 5% to 50 % (Casanova and Brock, 2000). The height of the wall at  
151 the end of the long section was modified in each experiment by taking the height  $h$  of  
152 the flooded area in order to avoid the reflection of the flood wave. The various  
153 combinations of these heights resulted in eighteen experimental runs. In twelve runs  
154 the peak flow developed in a vegetated section where after the peak flow event canopy  
155 was submerged always less than 50% of its plants height (in vegetated runs) and in the  
156 other six runs, it developed in a section without vegetation. Across the whole  
157 experiment, the variable  $H-h$  ranged between 3 and 9 cm, with the height  $h$  ranging from  
158 3 to 9 cm, in order to simulate low to high inundation levels. The condition  $H-h=0$  (i.e.  
159  $H=h$ ) was not considered since it would correspond to a gravity current, already studied  
160 (Soler et al., 2017, 2020, 2021), and corresponding to a different hydrodynamical  
161 process from a peak flow. Runs started once the lock gate was lifted and finished when  
162 the peak flow arrived at the end of the flume. In order to test for replicability, Run #5  
163 (Table 1) was repeated 3 times.



164

165 **Figure 1. a)** Side view of the laboratory flume, which is divided prior to the start of each  
 166 experimental run by a removable, sealing partition (lock gate) into two sections. The  
 167 smaller, left-hand, section is a reservoir for preparation of the turbidity current fluid.  
 168 The right-hand section contains the real or simulated vegetation and is the experimental  
 169 test section. Water depth in left-hand section is  $H$ , and in right-hand section is  $h$ . The  
 170 vertical coordinate is  $z$ , with  $z = 0$  at the bed (increasing upwards); the longitudinal  
 171 coordinate is  $x$ , with  $x = 0$  at the lock gate (increasing to the right). Thirteen sediment  
 172 traps (ST1 to ST13) on the flume bed. **b)** Images of samples of natural vegetation utilised  
 173 in the experiments: (a) *Juncus maritimus* and (b) *Arthrocnemum fruticosum*. (c) Top view  
 174 of the randomly-distributed array of obstacles used, with solid plant fraction (SPF) of  
 175 1.0%.

176

177

Run	Vegetation	H (cm)	h (cm)
1	<i>J.</i>	12	3
2	<i>maritimus</i>		6
3	(SPF 1%)		9
4		9	3
5			6
6		6	3
7	<i>A.</i>	12	3
8	<i>fruticosum</i>		6
9	(SPF 1%)		9
10		9	3
11			6
12		6	3
13	<i>Non-</i>	12	3
14	<i>vegetated</i>		6
15			9
16		9	3
17			6
18		6	3

178

179 **Table 1** List of experimental runs for vegetated and non-vegetated conditions for  
 180 different H (initial height) and h (inundated canopy height).

181

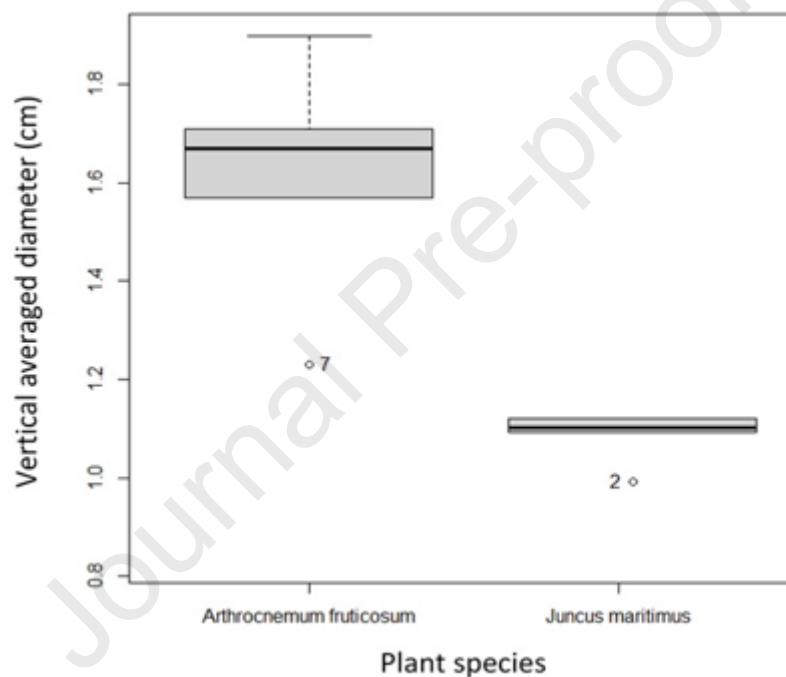
182 **2.2 Wetland vegetation characteristics**

183 The vegetation was positioned in the longer section of the flume (right hand side in Fig.  
184 1). Two species of natural vegetation were used: *Arthrocnemum fruticosum* and *Juncus*  
185 *maritimus*, both of which are native to Mediterranean salt marsh areas in marine inter-  
186 tidal environments and common in temperate climatic zones. Both species are  
187 characteristic of either inundated or non-inundated coastal wetlands (Batriu et al.,  
188 2011), reaching heights of 50 to 100 centimetres.

189 Plant individuals were collected from field sites in the Empordà marshes Natural Park  
190 (La Pletera), NE Spain. Sample plants with unusually high or low turgidity (judged  
191 subjectively) were not selected to maintain a standard set of plant characteristics. The  
192 plants within the vegetated section were randomly distributed by means of a random  
193 number generator, following Pujol et al. (2013). Plant density was quantified, following  
194 Pujol et al. (2010) using the solid plant fraction (SPF), which is defined as the fractional  
195 area at the bed occupied by the vegetation stems,  $SPF = 100N\pi(d/2)^2/A$ , where N is the  
196 total number of plants, A is the total bed area and d is the diameter of the plant stem.  
197 For all experiments with vegetation (experiments 1 to 12 in Table 1) a constant SPF of  
198 1.0% was used (Fig. 1B.3), corresponding to a canopy density of 356 obstacles  $m^{-2}$ , which  
199 is characteristic of coastal intertidal areas (Leonard and Luther, 1995).

200 The two types of vegetation presented different vertical distributions of frontal area for  
201 the same canopy density. *J. maritimus* is a rigid emergent plant with a slight vertical  
202 variation in its stem diameter (Fig. 1B.1). *A. fruticosum* is a rigid and emergent plant,  
203 which branches out over its height, each branch having leaves (Fig. 1B.2), resulting in  
204 wide vertical variation of its effective diameter. In order to quantify the frontal  
205 obstruction by each plant species, the vertical averaged plant diameter was determined

206 following the method described by Soler et al. (2020). The mean diameter  $d$  for the  
 207 stems of *A. fruticosum* and *J. Maritimus* was found to be  $1.6\pm 0.1$  cm and  $1.1\pm 0.1$  cm,  
 208 respectively. Data obtained for the vertical averaged diameter of the stems of *A.*  
 209 *fruticosum* were compared with those of *J. maritimus*. For their comparison, a one-  
 210 factor ANOVA was performed and significant differences between them were found ( $F$   
 211  $= 37.52$ ;  $p$ -value $<0.01$ ), see Figure 2.



212

213 **Figure 2.** Analysis one-factor ANOVA of the vertical plant diameter ( $d_z$ ) values for each  
 214 of the natural vegetation canopies: *Arthrocnemum fruticosum* at the left, and *Juncus*  
 215 *maritimus* at the right. Significant differences between them were found ( $F$ -value =  
 216  $37.52$ ,  $p$ -value  $<0.01$ ).

217

218 The vertical averaged stem diameter of each individual plant,  $d$ , and the value of the  
 219 frontal area of obstacles per unit volume ( $a = Nd/A$ , where  $A$  is the vegetated area

220 covered with N plant shoots) (Nepf, 1999) were combined to calculate the dimensionless  
221 array density,  $ad$ . This represents the volume of the vegetated area as a proportion of  
222 the total volume of the system, and was used as a single parameter to characterize the  
223 volumetric density of the vegetation. For the experimental runs with vegetation,  $ad$   
224 varied between 0.043 and 0.093, thus falling within the range observed in natural  
225 vegetation canopies (0.01 to 0.1, Kadlec, 1990; Soler et al., 2021).

226

### 227 **2.3 Sediment characteristics and measurements**

228 Thirteen square holes of 5 cm × 5 cm were created in the centre of the PVC base along  
229 the main axis of the flume to allocate sediment traps with the same size. The 13 traps  
230 (ST1 to ST13 in Fig.1A) were placed in these holes (being the same height as the base  
231 sheet, thus reducing their presence in the system) within the longer section of the flume,  
232 which were 15 cm from each other. When the flood arrived at the far end of the flume,  
233 the experimental run was deemed to have ended and the traps were covered with lids  
234 to collect the deposited sediment, and avoid any additional sediment being deposited  
235 by the reflected floodwater.

236 The simulation of a sediment-laden peak flow consisted of a homogeneous mixture of 3  
237 g/L of natural sediment in water, similar to that used for simulating sediment-laden  
238 gravity currents in lock gate experiments by Soler et al. (2020). The sediment used was  
239 taken from the Baix Empordà wetlands (La Pletera, NE Catalonia, Spain). Fragments of  
240 vegetation were removed from the sediment, which was then sieved to remove particles  
241 with diameter >500  $\mu\text{m}$  (see the Supplementary Material for the details of the  
242 sediment). A homogeneous mixture of initial concentration  $C_0=3\text{g/L}$  of sediment was

243 used for section A. For this, a sediment mixture (134.54 g of sediment) was prepared in  
244 a beaker (with 3 L of water taken from section A) and strongly stirred for a minimum of  
245 5 minutes. After this time, the volume of water and sediment was returned back into  
246 section A of the flume (Fig. 1A). Once the sediment was well mixed in the compartment,  
247 10 s passed before the vertical lock gate was lifted, and the experimental run started.

248 The depositional flux (DF) at each sediment trap for both fine and coarse sediment  
249 particles was calculated as the deposited mass per unit area and time over which the  
250 deposition occurred. The area considered was that of the sediment trap. The  
251 depositional flux (in  $\text{g cm}^{-2} \text{h}^{-1}$ ) was normalised by the initial horizontal flux of sediment  
252 carried by the current as it emerged from the reservoir ( $C_0v_0$ ), giving a non-dimensional  
253 depositional flux rate for each trap.

254

#### 255 **2.4 Determination of flood hydrodynamics**

256 Four stationary tripods were distributed along the experimental channel on which CCD  
257 cameras were mounted to record the experiments (Fig. 1A). The cameras were used to  
258 measure the horizontal evolution of the front of the peak flow (interface between the  
259 turbid flow and clean water) along the flume over time. The mean position of the peak  
260 flow front across the transversal axis of the flume was considered to determine its  
261 temporal evolution along the main axis. Parallax error was less than 4% in these images  
262 and was not corrected for in the analysis. The horizontal position of the peak flow front  
263 was monitored at time intervals of 0.2 s along the full development of the peak flow  
264 front.

265 In this study, the water height difference ( $H-h$ ), reduced gravity ( $g'$ ) and time ( $t$ ) were  
266 considered to be the main parameters to describe the behaviour of the peak flow. The  
267 reduced gravity is  $g'=g\cdot(\rho-\rho_o)/\rho_o$ , where  $g$  is the gravitational acceleration and  $\rho$  and  $\rho_o$   
268 are the densities of the sediment and water, respectively. For the early development of  
269 the peak flow in the non-vegetated experiments, the along-flume horizontal  
270 displacement ( $x$ ) of the peak flow front is expected to depend on the dimensionless  
271 parameter ( $g't^2/(H-h)$ ), that is the ratio of the gravitational inertia ( $g't^2$ ) and the potential  
272 inertia. For the vegetated experiments, the porosity of the vegetated zone ( $1-ad$ ) was  
273 also considered.

274 At the final stages of the peak flow process, when the water level became even all along  
275 the flume, the flow effectively became a gravity current. Thereafter, the governing  
276 parameter that drove the front evolution was the density difference between the front  
277 (with suspended particles) and the surrounding (clear) water. As found by Soler et al.  
278 (2017, 2020), a gravity current flowing along a flume passes through three regimes  
279 (inertial, drag and viscous regimes). The inertial regime, when position of the front,  $x_c$ ,  
280 varies in direct proportion to time,  $x_c \propto t^1$ , and depends on the reduced gravity and water  
281 depth (Tanino et al., 2005). When the gravity current is affected by the drag due to the  
282 vegetation, the temporal evolution of the front is  $x_c \propto t^{1/2}$  (Hatcher et al., 2000). And  
283 when viscosity dominates the gravity current development (viscous regime), the  
284 temporal evolution of the gravity current varies as  $x_c \sim t^{1/5}$ .

285

## 286 **3 RESULTS**

### 287 **3.1. Hydrodynamics of the peak flow**



288 The temporal evolution of the position of the peak flow front along the flume was  
 289 analysed to determine its relationship to the main parameters driving its behaviour.  
 290 Different hydrodynamic regimes were observed.

291

### 292 **3.1.1. Peak flow-adjustment regime**

293 The initial phase of the peak flow ( $x < 9.5(H-h)$ ) was identified as the peak flow-  
 294 adjustment regime. For all the experimental runs, both with and without vegetation, the  
 295 non-dimensional position of the peak flow,  $x/(H-h)$ , was found to have a statistically  
 296 significant linear relationship ( $r^2 = 0.85$ ;  $n = 84$ ;  $p < 0.05$ ) with  $g' \cdot t^2 / (H-h)$ , Figure 3,  
 297 following

$$298 \quad \frac{x}{H-h} = 13.04 \cdot \left( \frac{g' \cdot t^2}{H-h} \right)^{1/2} \quad (1)$$

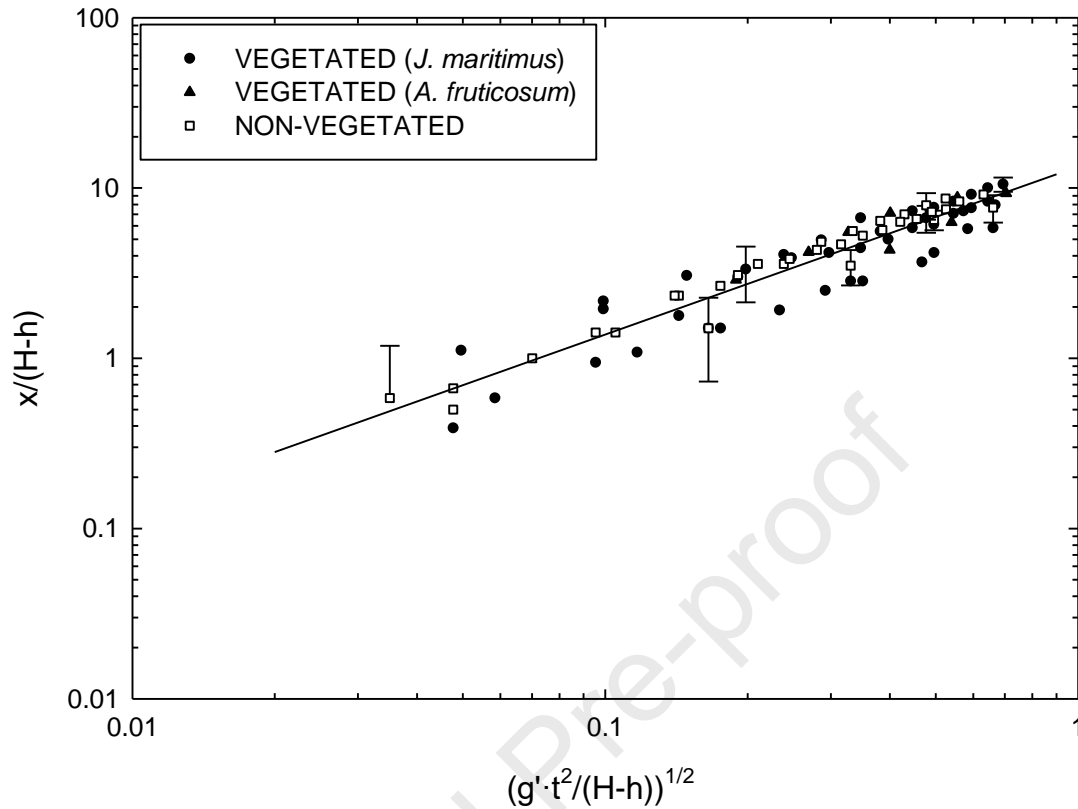
299 From equation (1), the position of the peak flow front followed:

$$300 \quad x = 13.04 \cdot [g' \cdot (H - h)]^{1/2} \cdot t, \quad (2)$$

301 and the velocity of the peak flow front during the flow adjustment,  $v_{PFA}$ , followed

$$302 \quad v_{PFA} = 13.04 \cdot [g' \cdot (H - h)]^{1/2} \quad (3)$$

303 Thus,  $v_{PFA}$  was found to be constant with time during this regime and dependent on  $g'$   
 304 and  $(H-h)$ .



305

306 **Figure 3.** Evolution, during the Peak flow-adjustment regime, of the dimensionless  
 307 length ( $x_c / (H-h)$ ) of the peak flow front versus the non-dimensional time  $(g' \cdot t^2 / (H-h))^{1/2}$   
 308 for all runs: vegetated (*Juncus maritimus* (black circles), *Arthrocnemum fruticosum*  
 309 (black triangles) and non-vegetated (white squares)). Line represents the linear best fit  
 310 of data ( $m = 13.04$ ;  $r^2 = 0.85$ ;  $n = 84$ ;  $p < 0.05$ ).

311

### 312 3.1.2. Peak flow regime

313 For distances  $9.5(H-h) < x < 27(H-h)$ , the flow transitioned to a fully developed peak flow.  
 314 In this regime, the peak flow behaviour depended on whether it developed in vegetated  
 315 or non-vegetated beds.

316 In non-vegetated beds, the temporal evolution of the position of the peak flow front  
 317 scaled as (see Figure 4A):

$$318 \quad \frac{x}{H-h} = 14.70 \cdot \left( \frac{g' \cdot t^2}{H-h} \right)^a \quad (4)$$

319 ( $r^2 = 0.89$ ;  $n = 105$ ;  $p < 0.05$ ), where  $a=1/2$ , which follows the same temporal evolution  
 320 as for the flow adjustment regime. In this case, the velocity of the peak flow,  $v_{PF}$ , was  
 321 found to be constant with time and to depend on  $g'$  and  $(H-h)$  according to:

$$322 \quad v_{PF} = 14.70 \cdot [g' \cdot (H-h)]^{\frac{1}{2}} \quad (5)$$

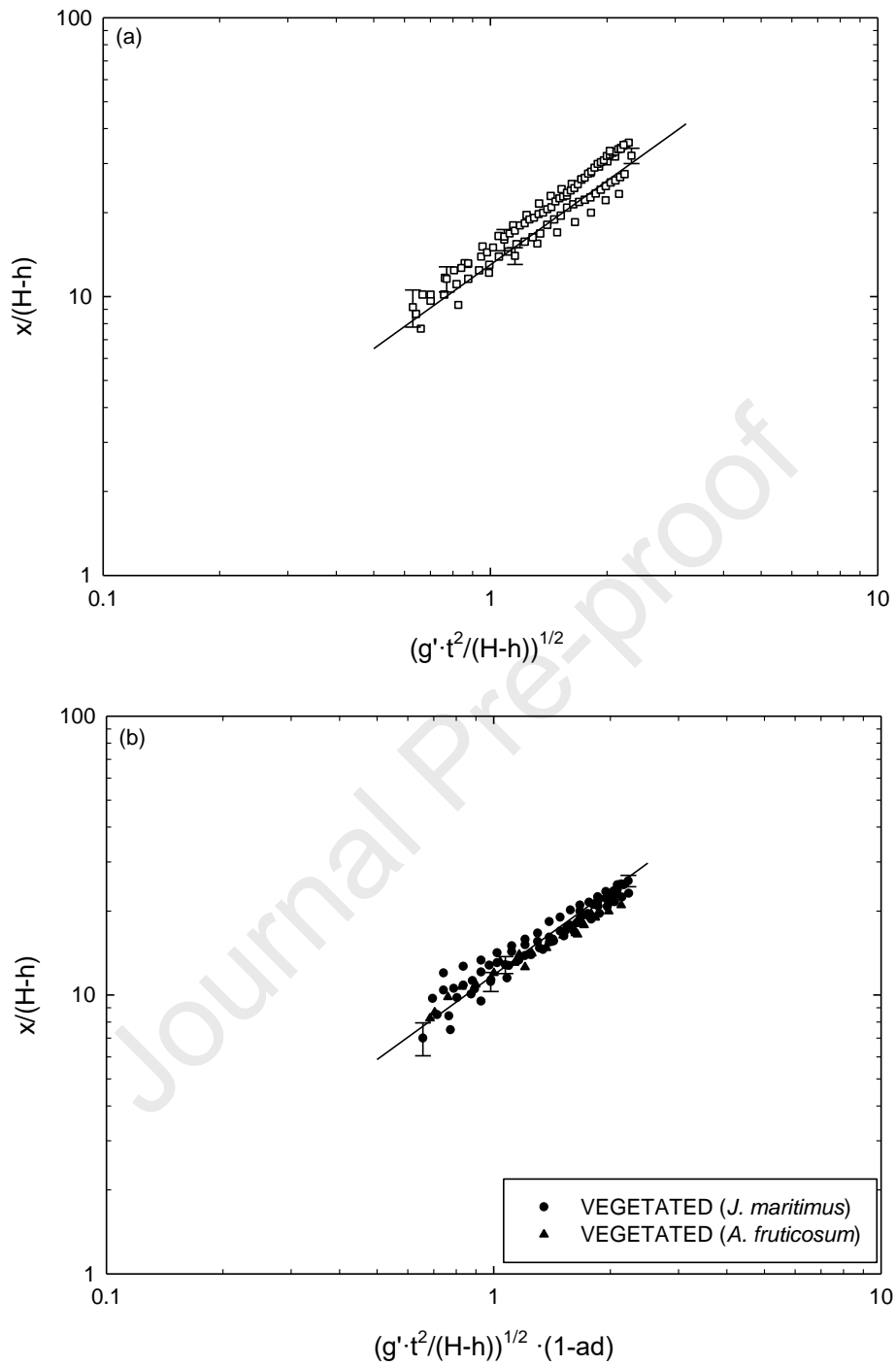
323 In contrast, in the vegetated beds, the drag exerted by the vegetation slowed the peak  
 324 flow front's temporal evolution along the flume within the vegetation in a manner  
 325 dependent on the non-dimensional ratio  $\left( \frac{g' \cdot t^2}{H-h} \right)$  and the dimensionless porosity of the  
 326 vegetation  $(1-ad)$ . The dependence of  $x/(H-h)$  on both these parameters was found  
 327 empirically to follow (Fig 4B)

$$328 \quad \frac{x}{H-h} = 10.06 \cdot \left( \frac{g' \cdot t^2}{H-h} \right)^b \cdot (1-ad)^c \quad (6)$$

329 ( $r^2 = 0.94$ ;  $n = 106$ ;  $p < 0.05$ ), where  $b=1/2$  and  $c=1$ . Consequently, the velocity of the  
 330 peak flow in the vegetated beds during this regime,  $v_{PFV}$ , was found to follow:

$$331 \quad v_{PFV} = 10.06 \cdot [g' \cdot (H-h)]^{\frac{1}{2}} \cdot (1-ad) \quad (7)$$

332 Therefore, like  $v_{FA}$  and  $v_{PF}$ ,  $v_{PFV}$  also remained constant with time and varied with  $g'$ ,  $(H-$   
 333  $h)$  and  $(1-ad)$ .



334

335 **Figure 4.** Evolution, during the Fully developed peak flow regime, of the dimensionless  
 336 length ( $x_c / (H-h)$ ) of the peak flow front versus (a) the non-dimensional time  $(g' \cdot t^2 / (H-$   
 337  $h))^{1/2}$  in non-vegetated runs, and (b) the non-dimensional time  $(g' \cdot t^2 / (H-h))^{1/2} \cdot (1-ad)$  in  
 338 runs with all canopies (*Juncus maritimus* (black circles) and *Arthrocnemum fruticosum*

339 (black triangles). Lines represents the linear best fit of data for both the non-vegetated  
 340 runs ( $m=14.70$ ;  $r^2 = 0.89$ ;  $n = 105$ ;  $p\text{-value} < 0.05$ ) and the vegetated experiments  
 341 ( $m=10.06$ ;  $r^2 = 0.94$ ;  $n = 106$ ;  $p\text{-value} < 0.05$ ).

342

### 343 **3.1.3. Peak flow drag dominated regime**

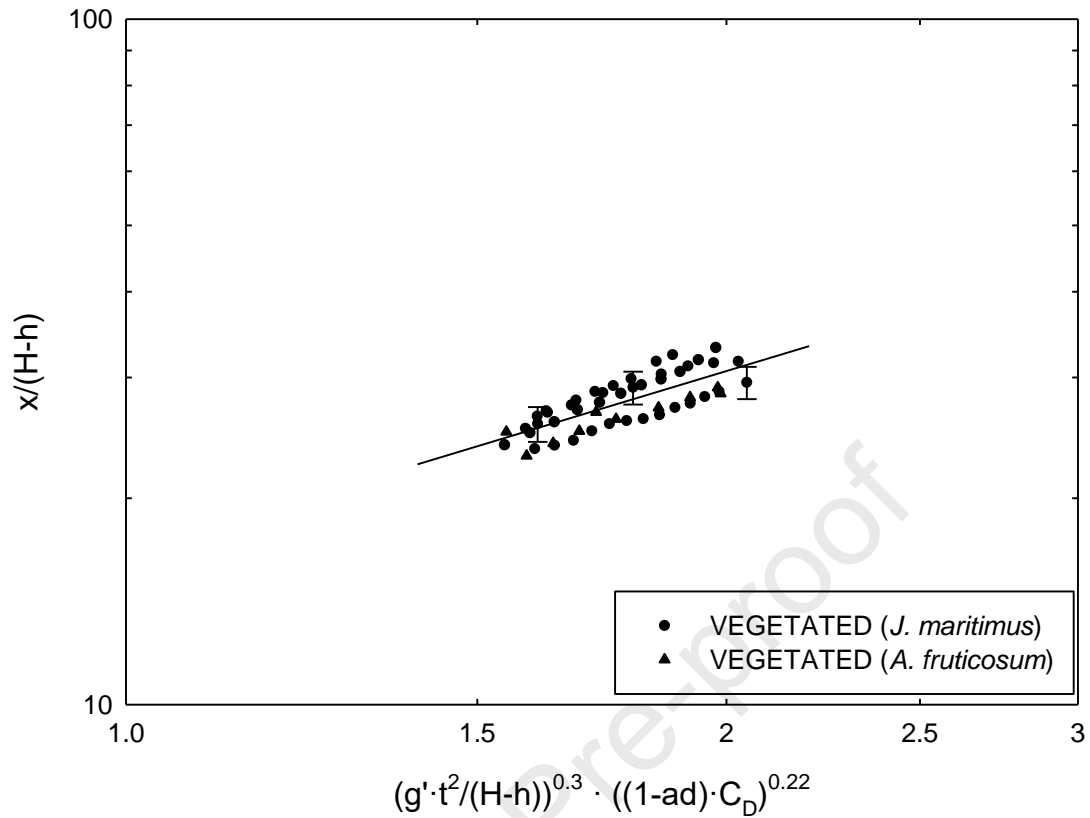
344 For distances,  $27 \cdot (H-h) < x < 32 \cdot (H-h)$ , the vegetation played a greater role and the  
 345 temporal evolution of the peak flow depended not only on the vegetation porosity ( $1-$   
 346  $ad$ ) but also on the drag coefficient of the randomly-distributed array.  $C_{Da} =$   
 347  $C_D / \{1.16[1.16 - 9.31(ad) + 38.6(ad)^2 - 59.8(ad)^3]\}$  is the drag coefficient (Ghisalberti and  
 348 Nepf, 2004), where  $C_D$  is the drag coefficient for smooth isolated circular cylinders (or  
 349 stems), which is a function of the stem Reynolds number,  $Re$ , such that  $C_D = 1 + 10Re^{-2/3}$   
 350 (White, 1991). This regime is named the peak flow drag dominated regime. Within this  
 351 regime, the evolution of the peak flow front (Figure 5) was found to follow

$$352 \quad \frac{x}{H-h} = 13.08 \cdot \left(\frac{g' \cdot t^2}{H-h}\right)^d \cdot (C_{Da}(1-ad))^e \quad (8)$$

353 ( $r^2 = 0.60$ ;  $n = 56$ ;  $p < 0.05$ ), where  $d=0.30$  and  $e=0.22$ . Therefore, the velocity,  $v_{PFD}$ , of  
 354 the peak flow followed:

$$355 \quad v_{PFD} = 7.85 \cdot (g')^{0.3} \cdot (H-h)^{0.70} \cdot (C_{Da}(1-ad))^{0.22} \cdot t^{-2/5} \quad (9)$$

356 Thus, in this regime, the peak flow velocity  $v_{PFD}$  varied not only with  $g'$ ,  $(H-h)$ ,  $(1-ad)$  and  
 357  $C_{Da}$  but also with time.



358

359 **Figure 5.** Evolution, during the peak flow drag dominated regime, of the dimensionless  
 360 length ( $x_c / (H-h)$ ) of the peak flow front versus the non-dimensional time  $(g' \cdot t^2 / (H-h))^{0.3} \cdot ((1-ad) \cdot C_D)^{0.22}$   
 361  $h)^{0.3} \cdot ((1-ad) \cdot C_D)^{0.22}$  in runs with all canopies (*Juncus maritimus* (black circles) and  
 362 *Arthrocnemum fruticosum* (black triangles). Line represents the linear best fit of data for  
 363 both the non-vegetated runs ( $m=13.08$ ;  $r^2 = 0.60$ ;  $n = 56$ ;  $p\text{-value} < 0.05$ ).

364

#### 365 3.1.4. Gravity current drag dominated regime and viscosity regime

366 For experiments with  $H-h \leq 6$ , and for distances  $x > 32(H-h)$ , the peak flow effectively  
 367 became a gravity current, and in this form first flowed through a drag-dominated gravity

368 current regime where  $x \sim t^{1/2}$ , and undergoing afterwards into a viscous regime of a  
369 gravity current where the front propagates as  $x \sim t^{1/5}$  (see Methods section 2.4).

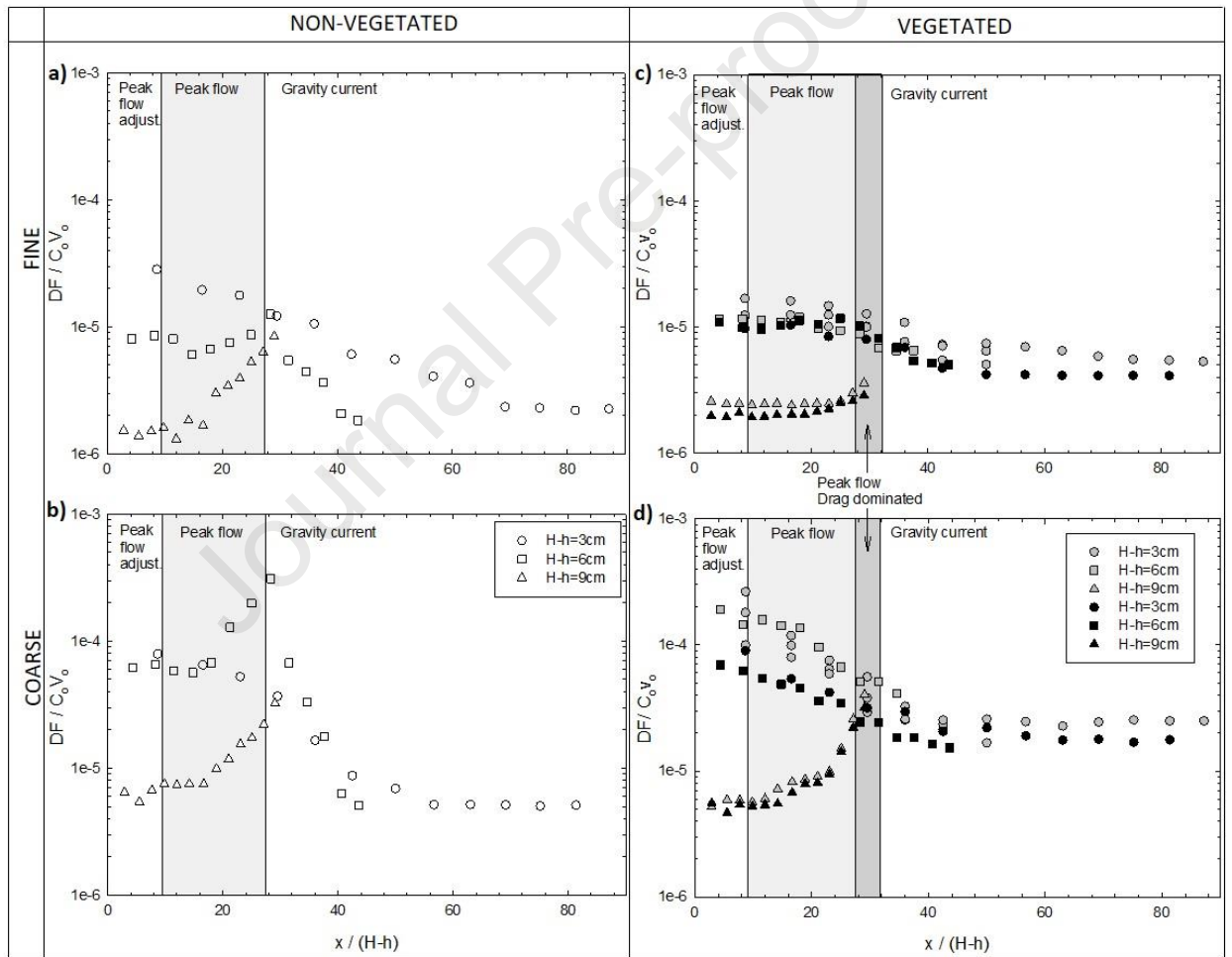
370

### 371 **3.2. Sediment deposition fluxes in a peak flow event**

372 For the non-vegetated runs, the non-dimensional deposition flux rates for both fine (Fig.  
373 6A) and coarse particles (Fig. 6B),  $DF/(C_o \cdot v_o)$  (where  $C_o$  was the initial sediment  
374 concentration ( $3 \text{ gL}^{-1}$ ) and  $v_o$  was the initial front velocity) was plotted against the non-  
375 dimensional distance,  $x/(H-h)$ . For the low inundation cases ( $H-h=9 \text{ cm}$ ), the non-  
376 dimensional depositional flux during the initial stages in the peak flow adjustment  
377 regime and the peak flow regime was constant, and reached its maximum at the end of  
378 the peak flow regime, with  $DF/(C_o \cdot v_o) \sim 10^{-5}$  and  $\sim 3 \times 10^{-5}$  for fine particles and coarse  
379 particles, respectively, at horizontal distances of  $x/(H-h) \sim 30$ . For moderate to high  
380 inundation cases ( $H-h = 3$  and  $6 \text{ cm}$ ), the behaviour of  $DF/(C_o \cdot v_o)$  with  $x/(H-h)$  depended  
381 on the inundation level for both fine and coarse particles. For instance, in the highest  
382 inundated case considered ( $H-h=3 \text{ cm}$ ), the non-dimensional depositional flux  $DF/(C_o \cdot v_o)$   
383 for both fine and coarse particles decreased with  $x/(H-h)$ , while for the intermediate  
384 inundation level of  $H-h=6 \text{ cm}$ , especially for the coarse fraction, the non-dimensional  
385 depositional flux was found to increase until the transition between the peak flow and  
386 gravity current regimes, as found for the low inundation regime of  $H-h=9 \text{ cm}$ .

387 For the vegetated runs and in low inundation experiments ( $H-h=9 \text{ cm}$ ),  $DF/(C_o \cdot v_o)$   
388 behaved similarly to the non-vegetated runs for both types of plant. In this case, the  
389 sedimentation rate was constant at the initial stage of the front development and  
390 reached a maximum value of  $\sim 4 \times 10^{-6}$  and  $\sim 4 \times 10^{-5}$  for fine particles (Fig 6C) and coarse

391 particles (Fig 6D), respectively, within the peak flow drag dominated regime. In contrast,  
 392 the sedimentation rates ( $DF/(C_o \cdot v_o)$ ) for moderate to high inundation vegetated  
 393 experiments ( $H-h = 6$  and  $3$  cm) remained nearly constant until the end of the peak flow  
 394 regime. After that,  $DF/(C_o \cdot v_o)$  decreased to the end of the flume, within the gravity  
 395 current regime. In the gravity current regime,  $DF/(C_o \cdot v_o)$  for fine particles was  
 396 independent of the vegetation type, whereas for coarse particles it was nearly 3 times  
 397 greater in runs with *J. maritimus* than in those with *A. fruticosum*.



398

399 **Figure 6.** Semi-logarithmic plot of the non-dimensional depositional sediment flux  
 400 ( $DF/C_o v_o$ ) against dimensionless downstream distance ( $x/(H-h)$ ). Left panels show results  
 401 for non-vegetated experiments and right panels for vegetated experiments. Both top

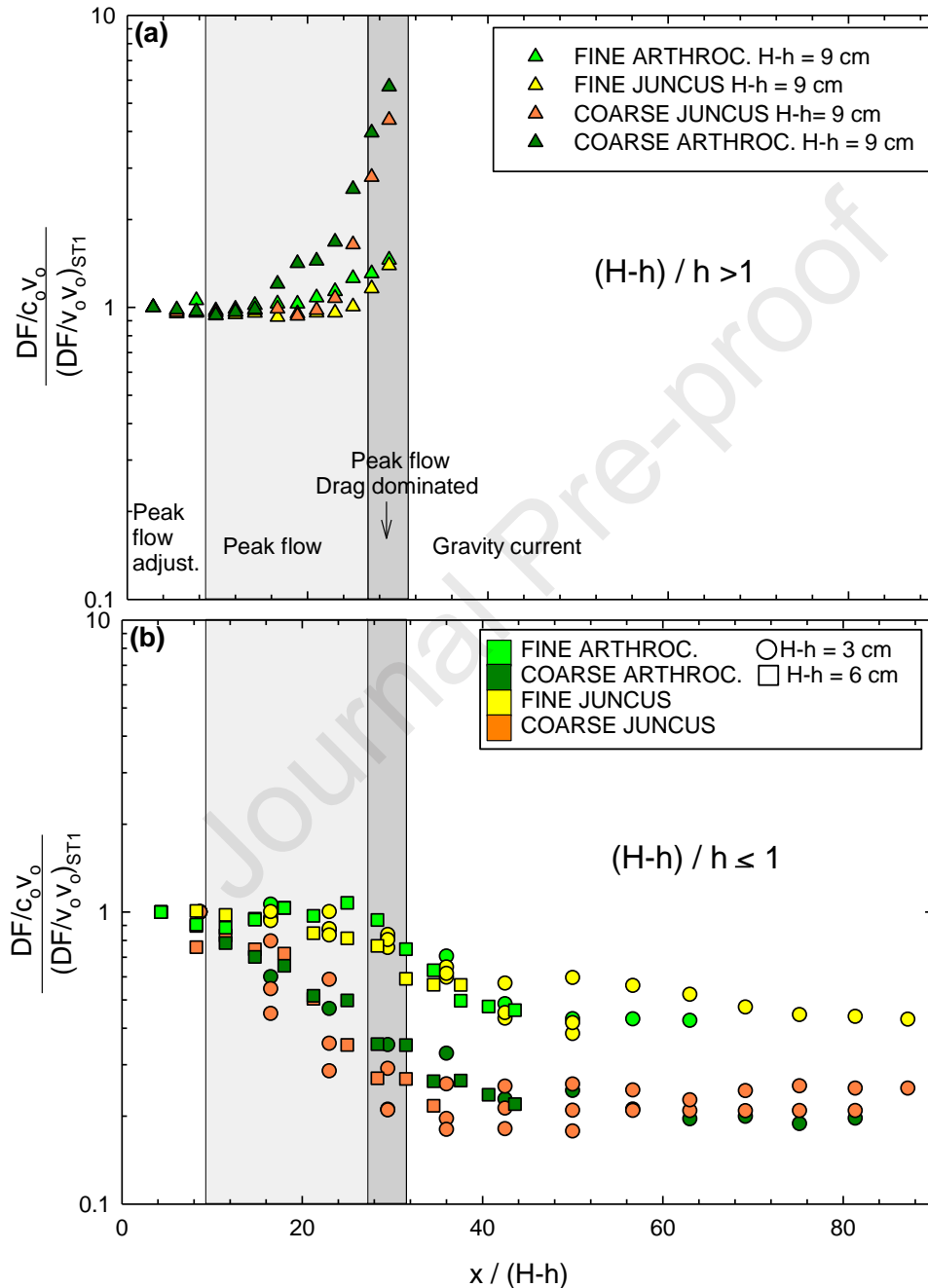


402 panels show fine sediment particles (particle diameters  $< 6.2 \mu\text{m}$ ) and the two bottom  
403 panels coarse sediment particles ( $6.2 \mu\text{m} < \text{particle diameter} < 104.0 \mu\text{m}$ ). All different  
404 levels on inundation, depending on (H-h) values, are shown in all the graphs: H-h=3 cm  
405 (circles), H-h=6 cm (squares) and H-h=9 cm (triangles). Left panel show non-vegetated  
406 experiment (with white symbols), and right panels show data for all canopies: *Juncus*  
407 *maritimus* (grey symbols), *Arthrocnemum fruticosum* (black symbols). The plots for non-  
408 vegetated runs are divided into three zones depending on the dynamical regime: flow  
409 adjustment, fully developed peak flow and gravity current. The plots for vegetated runs  
410 are divided into four zones depending on the dynamical regime: flow adjustment, fully  
411 developed peak flow, peak flow drag dominated and gravity current.

412

413 In order to study the effect of vegetation and the level of inundation on the transport of  
414 sediments, the non-dimensional depositional flux  $DF/(C_o \cdot v_o)$  was normalized by the  
415 value of the first sediment trap, ST1 (Figure 7). The inundation level parameter,  $(H-h)/h$ ,  
416 was used to distinguish between sedimentation patterns during the peak flow  
417 development.  $(H-h)/h > 1$  indicated low inundation levels (Fig. 7A) whereas  $(H-h)/h \leq 1$   
418 indicated moderate to high inundation levels (Fig. 7B). For low levels of inundation, the  
419 run-out profile of sedimentation was found during the peak flow and the peak flow drag  
420 dominated regimes for both types of vegetation, to have 4 times higher values for the  
421 coarse fraction than for the fine fraction (Figure 7A). For high levels of inundation, the  
422 sedimentation flux of fine particles was almost constant up to the peak flow regime for  
423 both types of vegetation and decayed essentially monotonically during both the peak  
424 flow drag dominated and the gravity current regimes (Figure 7B). The sedimentation  
425 flux of coarse particles for both types of vegetation was constant up to the mid-distances

426 during the peak flow regime and decayed thereafter. In the later stages of flow  
 427 development, during the gravity current regime, sedimentation rates in both types of  
 428 vegetation were constant.



429

430 **Figure 7.** Ratio of the logarithmic non-dimensional  $DF/C_0v_0$  between the trap at  $x$  and  
 431 the trap at  $ST1$  plotted against dimensionless downstream distance  $(x/(H-h))$  for (a) low

432 inundated canopies  $(H-h)/h > 1$ , that is  $H-h=9$  cm (triangles) and (b) high inundated  
433 canopies  $(H-h)/h \leq 1$ , that is  $H-h=3$  cm (circles) and  $H-h=6$  cm (squares). Values are  
434 differentiated depending on particles sizes: light colours (light green and yellow) refer  
435 to fine particles (particle diameters  $< 6.2 \mu\text{m}$ ), and dark colours (dark green and orange)  
436 to coarse particles ( $6.2 \mu\text{m} < \text{particle diameter} < 104.0 \mu\text{m}$ ). Data for all canopies: *Juncus*  
437 *maritimus* (yellow and orange symbols), *Arthrocnemum fruticosum* (green symbols) are  
438 shown. The plots are divided into four zones depending on the dynamical regime: flow  
439 adjustment, fully developed peak flow, peak flow drag dominated and gravity current.

440

#### 441 **4 DISCUSSION**

442 Pluvial, fluvial and coastal wetlands are known to mitigate the consequences of storms  
443 exacerbated by climate change by reducing peak flows during floods. The current study  
444 mimics in the laboratory the development of peak flows in inundated wetlands. The  
445 findings reveal that a peak flow passes through four different regimes, in which the level  
446 of the inundated zone (either with vegetation or not), and the vegetated plant  
447 properties are the key parameters that control its development.

448 Three identified regimes were found: the peak flow-adjustment regime, the peak flow  
449 regime and the peak flow drag dominated regime (in the vegetated cases). After these  
450 initial peak flow regimes, the flow became a gravity current undergoing into the well-  
451 known drag-dominated and viscous gravity current regimes (Hatcher et al., 2000; Soler  
452 et al., 2017).

453

#### 454 *4.1 The level of inundation impacting on the hydrodynamics of peak flow*

455 In inundated wetlands such as flooded floodplains, coastal wetlands and salt marshes,  
456 the storm magnitude (through the difference in the water level of the entering water  
457 compared to the level of the water initially inundating the wetland) is responsible for  
458 the development of the flow and the rate of sediment accretion. These results coincide  
459 with the fact that high accretion rates, driven by sedimentation in marsh zones subject  
460 to higher inundation levels, were found in the island of Sylt (Germany) after storms  
461 (Schuerch et al., 2012), or in the Nanhui tidal flat of the Changjiang Delta (China), where  
462 storms left clear signatures on tidal flat wetlands in both horizontal and vertical  
463 sedimentary features (Zhou et al., 2022).

464 This study has found that the level of inundation and the traits of the vegetation impact  
465 on the progression of the peak flow into the wetland canopy. For non-vegetated cases,  
466 in the initial development of the peak flow (beginning of the wetland) the velocity was  
467 dependent on  $(H-h)^{1/2}$ , while in the presence of vegetation the velocity was dependent  
468 on both  $(H-h)^{1/2}$  and  $(1-ad)$ . At the beginning of the progression of the flood, the  
469 vegetation affects only the level of the effective free path it leaves (i.e. the porosity of  
470 the vegetated area,  $1-ad$ ). That is, the greater the porosity the greater the velocity of  
471 the front of the peak flow. In contrast, at longer distances, the dependence of the  
472 progression of the peak flow front in the presence of vegetation changed to  $(H-h)^{0.7}$  with  
473 vegetation affecting the peak flow development not only through the porosity,  $1-ad$ , but  
474 also the canopy drag,  $C_{Da}$  and also due to the level of submergence of the vegetation.  
475 The last finding would agree with Jahaveri and Babbar-Sebens (2014) who found that  
476 deep wetlands were able to minimize peak flows more than the shallow ones, reducing  
477 them up to 20% or 11%, respectively. However, to our knowledge, few studies have  
478 reported the dependence of velocities of peak flows on both the inundation level and

479 the traits of vegetation in inundated wetlands. In this regime the velocity was a function  
480 of time,  $t^{-2/5}$ .

481

#### 482 *4.2 The level of inundation impacting on the sediment transport of peak flow*

483 If the level of inundation was low, i.e.,  $(H-h)/h > 1$ , vegetation did not control the  
484 sedimentation, with the sedimentation showing a typical run out profile during the peak  
485 flow development, with higher values during the peak flow drag regime for the coarse  
486 than for the fine particle range. Therefore, in low inundated and dry wetlands,  
487 sediments accumulate far from the source where net accumulation is expected to  
488 provoke a bed elevation. Then, in a hypothetical sequence of floods, this bed elevation  
489 may greatly reduce floodplain inundation that in turn may reduce downstream flood  
490 attenuation and increase downstream flood hazard (Guan et al., 2016). Therefore, dry  
491 wetlands would be more vulnerable in front of flooding events compared to inundated  
492 wetlands.

493 Besides, in low inundation conditions, and after the passage of the peak flow, the  
494 sedimentation fluxes showed that there was no segregation between the coarse and the  
495 fine fractions of sediment during the peak flow development, which for example can  
496 mediate significant amounts of sediment loads that are quickly deposited on riverbanks  
497 with sediments not being sorted hydrodynamically (Khurram et al., 2023). The lack of  
498 sorting of particle ranges, resemble flash flood partition of sediment load of arid and  
499 semiarid watersheds, in which over decades, the fluxes of the two fractions are  
500 approximately the same, with both fractions transported during small to moderate  
501 events (Malmon et al., 2004).

502 On the other hand, results demonstrate that marshes or wetlands with a high level of  
503 inundation, i.e.,  $(H-h)/h \leq 1$ , could control the transport of sediments, slowing the  
504 velocity of the peak flow and enhancing their sedimentation. Therefore, a wetland can  
505 be more effective controlling the transport and deposition of sediments carried by peak  
506 flows at a critical water level. At the beginning of the peak flow development along the  
507 inundated system, there is not substantial differential sedimentation of both fine and  
508 coarse particles with distance, but as peak flow enters in wetland canopy, and the  
509 vegetation effect increases, it is found that settling sediment is sorted by particle sizes.  
510 High levels of inundation can strongly reduce seedling and shoot development of some  
511 plants, with some of them preferring muddy substrates to clay substrates developed at  
512 high distances (van Riel et al., 2022). Schuerch et al. (2012) identified that a critical  
513 inundation height of 18 cm in salt marshes may determine the strength of accretion. In  
514 low marsh zones subject to higher inundation levels, mean storm strength is the major  
515 factor affecting marsh accretion, whereas in high marsh zones with lower inundation  
516 levels, it is the storm frequency that impacts marsh accretion (Schuerch et al., 2012).

517

#### 518 *4.3. Wetland vegetation effect on hydrodynamics and transport of sediments*

519 Our study has demonstrated that the presence of vegetation decreases the level of  
520 inundation needed for the system to be effective in enhancing the settling of sediment  
521 transported by the peak flow. Run out sedimentation profiles were observed for low  
522 inundation levels, which were enhanced by the presence of vegetation for both the fine  
523 and coarse particle fractions. In the initial stages of the peak flow, vegetation also  
524 modified the hydrodynamics of the peak flow, reducing the peak flow velocity as the

525 vegetation porosity decreased. At later stages, the modification was determined to be a  
526 function of both vegetation porosity and the drag coefficient.

527

528 Modification of peak flow velocity can be seen with the ratio between the non-  
529 dimensional velocity travelled by a peak flow event through a non-vegetated area (Eq.  
530 5) and that travelled over a vegetated areas (Eq. 7):

$$531 \frac{v_{veg}}{v_{non-veg}} = 0.68 \cdot (1 - ad) \quad (10)$$

532 Velocity in vegetated wetlands reduces directly proportional to the frontal area of plants  
533 stems per unit volume ( $ad$ ), that means that not only the vegetation density but also the  
534 morphologic characteristics of plants (with more or less leaves, thinner or wider stems,...)  
535 have to be considered in order to know the level of reduction in velocity peak flow.  
536 Wetland plant community and abundance of species will vary over time, as well as plants  
537 morphology. These changes will happen along the year (seasonal), impacting on the  
538 reduction on peak flow velocity by the wetland. Kadlec (1990) claimed that natural  
539 vegetation ranges between  $ad=0.01$  and  $0.1$  which corresponds to a range from 83 to  
540 826 plants/m<sup>2</sup> for *J. maritimus*, or from 39 to 391 plants/m<sup>2</sup> for *A. fruticosum* (natural  
541 plants taken for performing the experiments). Therefore, depending on the vegetation  
542 morphology, wetlands could reduce the velocity from 32% to 39% (corresponding to  
543 thicker and thinner plant stems, respectively).

544 Vegetation not only has an effect on velocity but also in the distance reached by the  
545 peak flow once enters in the wetland. Therefore, as found in equation (10) this study  
546 demonstrates that the ratio between the non-dimensional distance travelled by a peak

547 flow event through a non-vegetated area (Eq. 4) and that travelled over a vegetated  
 548 areas (Eq. 6), is

$$549 \frac{\left(\frac{x}{H-h}\right)_{veg}}{\left(\frac{x}{H-h}\right)_{non-veg}} = 0.68 \cdot (1 - ad) \quad (11)$$

550 indicating that for canopies with a greater frontal area of plants, peak flow arrives a 20%  
 551 further than for non-vegetated wetlands.

552 However, once a wetland is vegetated, the density of vegetation plays a minor role. For  
 553 example, comparing the densest to the sparsest canopies that can be found in natural  
 554 vegetation (which following Kadlec (1990), and taking a typical wetland plant as *Spartina*  
 555 *alterniflora*, would correspond to 69 to 694 plants/m<sup>2</sup>), it results in,

$$556 \frac{\left(\frac{x}{H-h}\right)_{veg, ad=0.1}}{\left(\frac{x}{H-h}\right)_{veg, ad=0.1}} = (1 - ad)^{0.22} = 0.98, \quad (12)$$

557 which means that denser canopies only reduce by a 2% the extension reached by the  
 558 peak flow flowing through sparser canopies.

559 From equation for the distance travelled by a peak flow event over a vegetated area (see  
 560 Eq. 8) :

$$561 x_{veg} = 13.8(g't^2)^{0.3} \cdot (H - h)^{0.7} \cdot (C_{Da}(1 - ad))^{0.22} \quad (13)$$

562 the first term on the right-hand side can be related to the effect of the submergence  
 563 level of the vegetated stems, whereas the second can be attributed to the effect of both  
 564 the drag and the density of the vegetation. Considering the effect on the peak flow  
 565 entrance of high inundated vegetated wetlands to low inundated vegetated wetlands  
 566 (H-h=9 and 3 cm respectively) it is found a ratio  $(x)_{INUNDATED}/(x)_{NON-INUNDATED}$  of 0.46. This



567 result indicates that the presence of vegetation reduces the extension of the peak flow  
568 to approximately half its value for the non-inundated case. This result is in accordance  
569 with Fairchild et al. (2021) who found a reduction in the flooded area of  $\sim 0.46$  for low  
570 storm magnitudes and 0.68 for high storm magnitudes. As well as with studies realized  
571 by De Laney (1995) that found a reduction in peak flow due to construction of wetlands  
572 concluding that 5%–10% of the wetlands area in the watershed could attenuate around  
573 50% of peak floods, and a little bit greater than the 42% found in Eagle Creek watershed  
574 (Indiana, USA) (Javaheri and Babbar Sebens, 2014).

575 In addition, the current study demonstrates that the higher the peak flow magnitude  
576 (i.e.,  $H-h$ ), the higher the ratio  $(x/(H-h))_{veg}/(x/(H-h))_{non-veg}$ . This result indicates that  
577 vegetation has a smaller capacity to attenuate deeper flooding events than shallower  
578 ones.

579 In the peak flow drag dominated regime, the reduction of velocity along with the  
580 increased sediment deposition depended on the vegetation. The higher frontal area of  
581 *A. fruticosum* could induce lower sedimentation of coarser particles associated with  
582 higher flow velocities through the vegetation (Serra et al., 2017, 2021). For high  
583 inundation levels, at shorter distances of flow development the coarse and fine  
584 sediment did not show differential settling with distance. At intermediate and large  
585 distances, the sedimentation profile with distance presented a monotonically  
586 decreasing development that was not affected by the type of vegetation, with the  
587 sedimentation of fine particles being higher than the sedimentation of coarse particles.  
588 This process has also been identified in the sedimentation of particles in gravity currents,

589 and is known as the “muddification” process, in which the presence of fine particles in  
590 deposited sediments is higher (Soler et al., 2020).

591

#### 592 *4.3 Wetland control on floodings*

593 In the ephemeral and dry channels, the development of a high peak flow can be very  
594 hazardous and damaging, with many reported inundation events, resulting in large  
595 amounts of sediment mobilisation and high rates of sedimentation (Camarasa, 2016;  
596 Hooke, 2009). The characteristics of these ephemeral channels are the lack of channel  
597 bed armour, high sediment supply, and equal mobility of sediment sizes. Due to climate  
598 change, wetlands will experience drought cycles more frequent and severe in the future  
599 (Middleton and Kleinebecker, 2012), that will become in a reduction of vegetation  
600 (Jenkins and Boulton, 2007). Consequently, wetlands will become more vulnerable to  
601 flood events. Studies carried out by Longobardi et al. (2003) across different countries  
602 (Australia, New Zealand and Italy) have shown that the ratio between the quick peak  
603 flow and the rainfall volume (run off coefficient) is dependent on the soil moisture prior  
604 to the event, anticipating higher peak flows and associated floodings for dryer wetlands.  
605 But not only this, sedimentation patterns are expected to differ between dry and  
606 inundated wetlands. Therefore, the future fate of the bed elevation and morphometry  
607 will also change differently in front of the inundation level after facing a flooding event.  
608 Vegetation, through the vegetation porosity and canopy drag, altered the  
609 hydrodynamical development of the peak flow and the associated transport and  
610 sediment depositional patterns. The results regarding both peak flow development and  
611 sediment transport showed that the hydrodynamic, morphodynamic and ecological

612 processes in floodplains, coastal wetlands and marshes may present both spatial and  
613 evolutionary characteristics that are governed by both the traits of the vegetation and  
614 the level of inundation in the system, at short time scales (Guan et al., 2016; Tsoi et al.,  
615 2022). Among all wetland services, flood control and climate change mitigation are the  
616 most important services for the human communities (Mitsch and Gosselink, 2007) so it  
617 is necessary to know the critical water level of inundated systems at which coastal  
618 wetland systems, can sustain most of its services.

619 In the period of 1998-2017 floods affected more than 2 billion people (UNISDR, 2018)  
620 causing considerable economic losses that can increase due to the anthropogenic  
621 warming. For example, for a mean global air temperature increase of 1.5 °C, and  
622 depending on the socio-economic scenario, human losses from flooding could rise by  
623 70–83% (Dottori et al., 2018). Therefore, it might be of great interest understanding the  
624 hydrological services provided by wetlands to face these future scenarios as they have  
625 great potential to be used as nature-based solutions for regulation of pluvial, fluvial or  
626 coastal floodings (Rojas et al., 2022). Therefore, conservation practices in wetlands  
627 should be fuelled to fight against peak flows in coastal areas. Results found show that  
628 densest vegetated wetlands would be very effective in reducing the circulation of a peak  
629 flow, not only by reducing the velocity between a 35 or 39% but also reducing a 20% the  
630 peak flow fetch within the wetland. Furthermore, inundated wetlands are expected to  
631 be even more effective than dry wetlands, raising the protection benefits of a wetland  
632 to a 50%.

633

634 **5 CONCLUSION**

635 Under flooding processes, inundated systems with vegetation may or may not control  
636 the hydrodynamics and sediment deposition with distance depending on the strength  
637 of the flooding event. In this study, the development of particle-laden peak flow has  
638 been studied in systems that can be subject to both low and high inundation conditions.  
639 The longitudinal evolution of the peak flow front was characterised by three temporal  
640 regimes: firstly, the peak flow-adjustment regime, then the established peak flow  
641 regime, and finally the peak flow drag dominated regime. At larger distances, the flow  
642 developed into a gravity current and evolved further through drag-dominated and  
643 viscous gravity current regimes. Sediment transport and depositional flux rates  
644 associated to the flooding event presented patterns that depended also on the  
645 inundation level. High inundations were able to transport sediment particles inland into  
646 the marsh whereas in low inundation levels the sedimentation was greater close to the  
647 source decreasing progressively into the marsh. Vegetation affected the peak flow in its  
648 early stages only by reducing the cross-section of the flow but keeping the non-  
649 dimensional flow velocity constant. However, as the peak flow developed further, the  
650 plants produced drag forces on the flow in the peak flow drag regime where the velocity  
651 of the flow decreased with time.

652 To summarise, this study investigated the hydrodynamics and sediment deposition  
653 during the beginning of flooding events (peak flow) and reports the variation of velocity  
654 with distance, finding it to depend on inundation levels, reduced gravity, vegetation  
655 porosity, vegetation drag, and time. It also investigated the longitudinal profile of  
656 sediment deposition under low and high inundation levels in the presence of vegetation.  
657 The results may help to understand the impacts of extreme pluvial, fluvial and coastal  
658 flooding events in vegetated floodplains, and wetlands, and can be applied to coastal

659 wetland flood risk management with the purpose of mitigating and fighting against peak  
660 flows.

661

#### 662 **Credit authorship contribution statement**

663 **Marianna Soler:** Conceptualization, Data curation, Formal analysis, Methodology,  
664 Writing-original draft, Writing-review & editing. **Jordi Colomer:** Conceptualization, Data  
665 curation, Formal analysis, Writing-review & editing. **Andrew Folkard:** Data curation,  
666 Writing-review & editing. **Teresa Serra:** Conceptualization, Writing- review & editing,  
667 Funding acquisition, Supervision.

668

#### 669 **Data availability**

670 Data will be available upon request to the first author of this paper.

671

#### 672 **Declaration of conflict of interest**

673 The authors declare that they have no known conflict of interest or personal  
674 relationships that could have appeared to influence the work reported in this paper.

675

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**Declaration of interests**

The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

The authors declare the following financial interests/personal relationships which may be considered as potential competing interests:

Journal Pre-proof